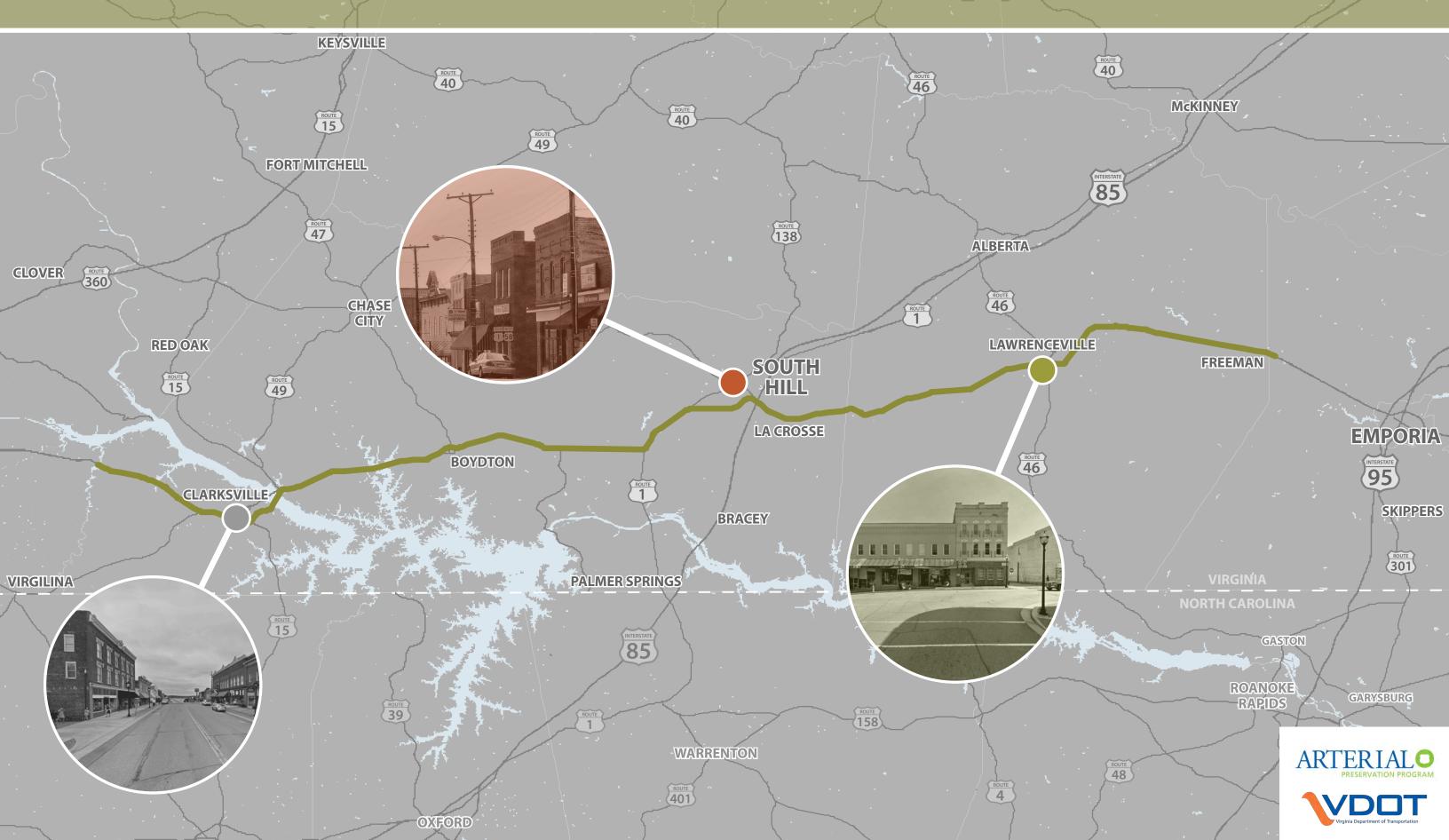
US 58 ARTERIAL PRESERVATION



US 58 Arterial Preservation Plan

Brunswick and Mecklenburg Counties

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Prepared for:



Virginia Department of Transportation

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LIST OF ACRONYMS

ADT	Average Daily Traffic
HCM	Highway Capacity Manua
_OS	Level of Service
PDC	Planning District Commission
PSI	Potential for Safety Improvement
ΓΑΖ	Traffic Analysis Zone
ГМРD	Transportation and Mobility Planning Division
гоsam	Traffic Operations and Safety Analysis Manua
ГТR	Travel Time Ratio
/DOT	Virginia Department of Transportation

CHAPTER 1: INTRODUCTION

1.1 Study Purpose
The purpose of the US 58 Arterial Preservation Plan is to develop a holistic approach that identifies ways to ensure the safety and preserve the capacity of the US 58 study corridor without wide-scale roadway widenings or increased signal proliferation. This Arterial Preservation Plan has been requested to identify investment recommendations that will help preserve and enhance this key transportation corridor due to the important role it plays in the region as a key freight corridor serving the Port of Virginia, a vital link within the Commonwealth, and a key facility for connections to North Carolina and points south.

1.2 What is the Arterial Preservation Program?
The Virginia Department of Transportation's (VDOT) Arterial Preservation Program is designed to preserve and enhance the capacity and safety of the critical transportation highways in Virginia. These major highways accommodate the long-distance mobility of people and goods throughout the Commonwealth. Preserving mobility on these corridors is critical to the current and future economy.

Within the framework of the Arterial Preservation Program, VDOT is developing methodologies to consistently and programmatically evaluate the corridors, creating a toolbox of preservation and enhancement strategies and identifying opportunities to implement these strategies. As an alternative to widening major highways to add capacity, preservation and enhancement strategies promote the use of innovative transportation solutions, minimizing delays for through traffic and improving safety, while incorporating local economic development goals. Developed in partnership with localities, the strategies are used as tools to plan for infrastructure that supports future land use and development.

1.3 Study Area

The study area, located in VDOT's Richmond construction district, traverses Brunswick and Mecklenburg Counties and extends from the Hampton Roads construction district boundary at the Greensville / Brunswick corporate limits to the western termini at the Lynchburg construction district boundary at the Mecklenburg / Halifax corporate limits. The study area is 65.7 miles in length. Figure 1 depicts the study area for the US 58 Arterial Preservation Plan.

1.4 Review of Existing Studies and Documents
A literature review gathered data and documented any proposed developments or projects for the US 58 corridor within the study area. These documents assisted in the development of land use assumptions



Figure 1. Study Area

and growth patterns and helped identify potential problem areas along the US 58 corridor. The literature review included the comprehensive plans for each locality in the study area, the Six-Year Improvement Plan, long range transportation plans, and corridor studies as noted below:

- VTrans2040
- Southside Planning District Commission (PDC) 2035 Regional Long Range Transportation Plan
- Brunswick County Comprehensive Plan
- Mecklenburg County Long Range Plan
- Town of Boydton Comprehensive Plan
- Town of Clarksville Comprehensive Plan
- Town of South Hill Comprehensive Plan
- US 58 Corridor Study South Hill, VA La Crosse, VA (VDOT)

1.5 Public Involvement Process

The public involvement process began with the April 17th, 2018 project kick-off/scoping meeting and subsequent discussion with the core study team. Project stakeholders involved in the development of the study included:

- Brunswick County
- Mecklenburg County
- Town of Boydton
- Town of Brodnax
- Town of ClarksvilleTown of LaCrosse
- Town of Lawrenceville
- Town of South Hill
- Southside PDC
- VDOT at the Residency, District, and Central Office level

This stakeholder group consisted of staff-level representatives from each of the identified organizations. This group met at key milestones throughout the study to review progress and results. These meetings were held at the Southside PDC offices located at 200 S. Mecklenburg Avenue in the Town of South Hill. Table 1 lists the dates and topics of these meetings.

Table 1. Core Study Team Meetings

Meeting Date	Meeting Topic
April 17, 2018	Study Kick-Off/Orientation
September 18, 2018	Existing Conditions/Opportunities for Improvement
May 20, 2019	Preliminary Study Recommendations
August 7, 2019	Final Study Recommendations

1.5.1 Stakeholder Surveys

As part of the outreach process, a web-based survey was conducted with study stakeholders in the summer of 2018 to understand current issues along the corridor and possible changes to the land use and local plans in the study area. Respondents also ranked highly-needed improvements in the corridor. Table 2 presents a summary of responses received.

Table 2. Stakeholder Survey Responses

Current Issues Along the Study Corridor (Not Ranked)
Safety
Allow easier access to businesses
Corridor needs updating to current standards
Need improved / additional turning lanes
Highly Needed Improvements (Ranked)
1. Access improvements
2. Safety improvements
3. Operational improvements
4. Geometric improvements

1.5.2 Public Outreach

A public meeting was held on January 29, 2019 at Southside PDC to review the existing conditions assessment and opportunities for improvements along the US 58 corridor. Eighteen citizens and stakeholders attended this meeting.

Members of the public were invited to provide comments on the preliminary findings and to suggest additional locations where improvements should be considered. Feedback received from the public was further reviewed during the recommendations' development process.

General comments received at the public meetings included:

- Concerns with truck traffic
- Concerns with vehicle speeds
- Need to improve median crossovers and add turn lanes
- Need to update corridor to current design standards
- Poor visibility at crossovers

Specific areas of concern from the public meeting included:

- Cattail Drive and Twin Ponds No turn lanes and dangerous crossing maneuver
- Crashes from Totaro Creek to US 46
- Dangerous turning movements around Brunswick Square
- Speed limit not observed in Brodnax
- I-85 in South Hill to La Crosse needs attention
- Turn lane improvements and acceleration lanes on US 58 in Boydton

A second and final public meeting was held on September 4, 2019 at Southside PDC to present the final corridor recommendations. The meeting included a formal presentation from the study team, various displays describing the study results, recommendations, and a citizen comment area. Twenty citizens and stakeholders attended the second public meeting. No written comments were submitted by the public in response to the final study recommendations.

CHAPTER 2: EXISTING CONDITIONS

2.1 Existing Land Use

The study area traverses miles of rural land, occasionally passing by the edge of a small town or serving as a major corridor for larger population centers' suburbs. The landscape is primarily agricultural or wooded in land use. The rural portions of the corridor feature large lot residential uses, large-scale industrial uses, and institutional uses. At major crossroads, low-density small-town development is likely and includes smaller lot residential uses as well as small-scale commercial and industrial. Further East, approaching Lawrenceville and South Hill, the study area becomes suburban in character, becoming the primary access route for regionally-significant commercial centers.

Existing Land Use Key Findings:

- Clusters of single-family homes with direct access to US 58:
 - In the Town of Brodnax:
 - In the Town of La Crosse;
 - Between Park View Circle and US 1 in Mecklenburg County; and
 - Between Carters Point Road and Buffalo Springs Road in Mecklenburg County.
- Retail development with direct access to US 58:
 - Brunswick Square in Lawrenceville;
 - In the Town of La Crosse; and
 - In the Town of South Hill.
- Industrial development with direct access to US 58:
 - Dominion Power, Brunswick County;
 - Redland Brick, Brunswick County;
 - Scotts, Brunswick County;
 - Brodnax Lumber, Brunswick County; and
 - Microsoft Data Center, Town of Boydton.
- Institutional uses with direct access to US 58:
 - Park View High School, Mecklenburg County; and
 - Park View Middle School, Mecklenburg County.
- Other relevant development with direct access to US 58:
 - Lawrenceville-Brunswick Municipal Airport.

2.2 Existing Infrastructure

A field review was conducted on June 12, 2018 at the outset of the study to review roadway and intersection configurations, identify deficiencies and areas of concern including sight distances or grade issues, identify unique roadway features, and observe traffic operations. US 58 is primarily a four-lane roadway running east-west and includes an interchange with Interstate 85 (I-85). The US 58 study corridor intersects with US 1 and US 15 in Mecklenburg County. Access along US 58 is primarily uncontrolled within the study area. The only sections along the corridor where access is fully or partially controlled are between US 58 Bus and US 15 in Mecklenburg, around Clarksville, and between Route 46 (Christanna Highway) and Route 641 (Bright Leaf Road). A full description of the field review for the corridor is available in Appendix B.

The corridor has several roadway segments with design features that may reduce capacity, level of service or safety. The western end of the corridor is characterized by numerous intersections and crossovers with sub-standard turn lanes. Often, significant grade differentials exist between the eastbound and westbound lanes of US 58 at intersections and crossovers. At Route 92 (Washington Street) and Route 4 (Buggs Island Road), limited sight distance impairs turning movements. Between the western US 1 intersection and Route 780 (Theater Road), US 58 has a two-way left turn lane accompanied by a noted increase in direct access points to US 58.

The highest intensity of development along the corridor is located in South Hill due to the presence of the I-85 interchange. The interchange suffers from adjacent roadways in close proximity, improper pavement markings for the southbound I-85 to the westbound US 58 through movement, and no turn lane or taper for westbound US 58 to northbound I-85. On the eastern side of the interchange, motorists were observed cutting across eastbound US 58 from the northbound I-85 off-ramp to turn left in a distance of less than 600 feet. Numerous access points and median crossovers accompanied by significant grade differences between lanes complicate maneuvers in this area.

East along the corridor, heading toward Brodnax, shoulder widths and shoulder types become inconsistent and crossovers lack turning lanes. US 58 in Brodnax is characterized by a continuous two-way left turn lane with frequent access points and narrow shoulders. The raised median resumes east of Brodnax, but several crossovers lack turn lanes and have poor sight distance. From Route 46 (Christanna Highway) to Route 641 (Bright Leaf Road), US 58 is primarily limited access. Route 641 is a skewed intersection that may be difficult for trucks to navigate. The pattern of frequent crossovers with insufficient turn lanes continues across the rolling terrain to the eastern termini of the study corridor. The results of the full inventory field review are available in Appendix C.



Figure 2. The Intersection of Route 58 and I-85 in South Hill

2.3 Existing Access
The number of crossovers such as intersections and median crossovers, points along the US 58 corridor were inventoried and the distance between each point measured and reviewed for compliance with VDOT's Access Management Spacing Standards which takes into account functional classification, roadway speed, and access type.

As identified in Table 3 and Figure 3, the evaluation of crossovers shows that only 54% percent (2.8 mi) of westbound segments and 32% percent (2.4 mi) of eastbound segments in the study corridor are noncompliant. The most significant areas of non compliance are in the Town of South Hill, and near the Town of Lawrenceville.

Figures 4 through 7 present a comprehensive inventory of access points and crossovers along the study corridor.

Table 3. Crossover Points Findings*

		Crossover Points									
_		Compliant	Non-Compliant	Total							
	Eastbound	39	18	57							
	Westbound	25	29	54							
	Total	64	47	111							

^{*}Compliance was calculated based on VDOT design standards, Table 2-2 of the Virginia Road Design Manual Appendix E, for access management of entrances and intersections.

Figure 3. Crossover Locations: Of 111 total crossover locations, 64 meet VDOT spacing requirements

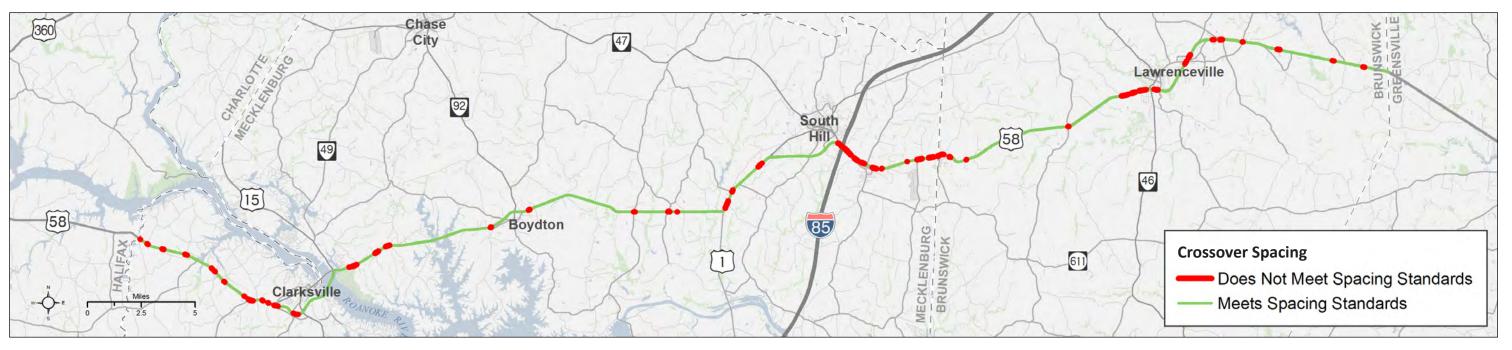


Figure 4. Eastbound Access Points, Western Half of Study Area

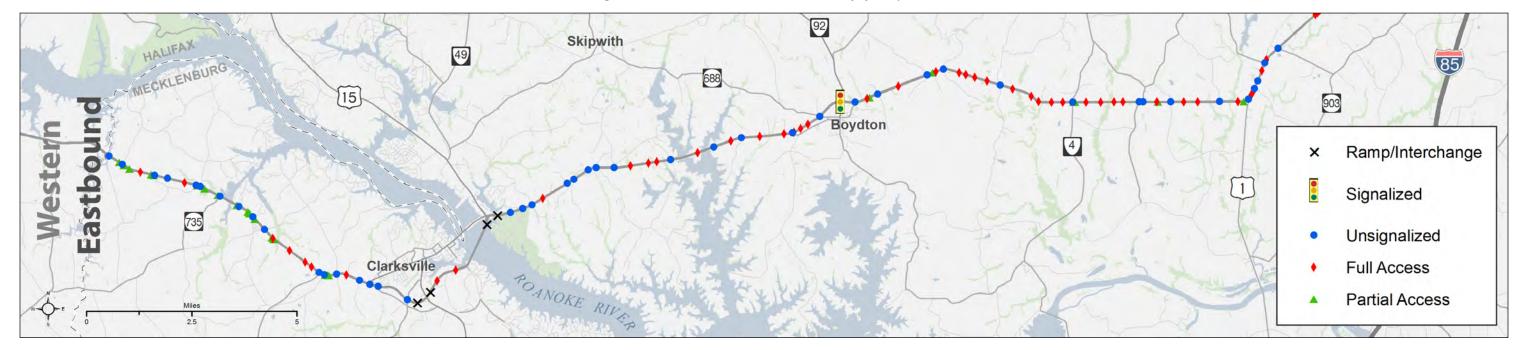


Figure 5. Westbound Access Points, Western Half of Study Area

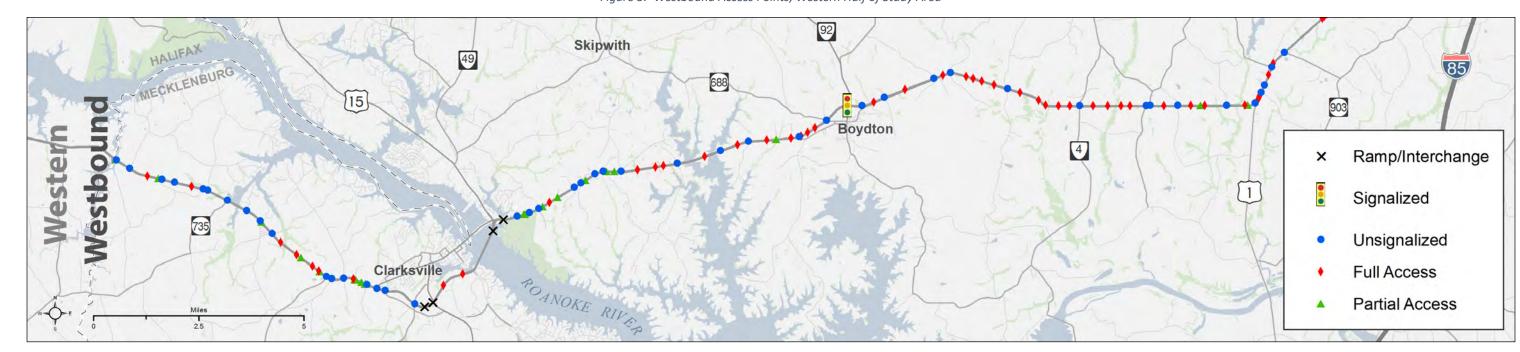


Figure 6. Eastbound Access Points, Eastern Half of Study Area

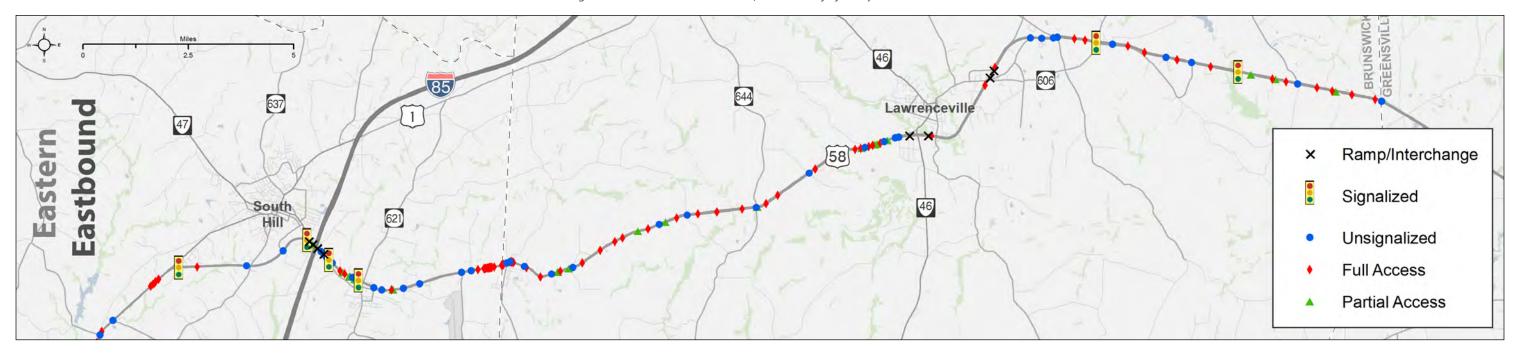


Figure 7. Westbound Access Points, Eastern Half of Study Area



2.4 Crash Analysis

An evaluation of corridor safety was conducted based on an analysis of crash information. The latest five years of available crash data (2013 to 2018) was obtained from VDOT's Roadway Network System to identify potential locations for safety improvements.

Analysis of existing conditions found that the crash rate for over 80% of the corridor is at or below the statewide average for a rural arterial. Portions of the corridor with crash rates that are greater than 100% above the statewide average are near or within the Towns of Lawrenceville and South Hill as well as near the western Mecklenburg County Line. Figure 10 illustrates the crash rate within the study area, Figure 11 illustrates the crash density within the study area, and Figure 12 illustrates the crash severity within the study area.

Crash Analysis Key Findings:

- 845 total crashes were reported between 2013 and 2018 along the study corridor. In 69% of crashes only property damage occurred with no injuries or fatalities. 2% of crashes resulted in fatal injury.
- The greatest number of crashes were fixed-object, off-road collisions, which accounted for 32.0% of crashes. This is followed closely by angle collisions, which accounted for 21.5% of crashes.
- The crash rate is highest in the Town of South Hill near the I-85 interchange.

In accordance with VDOT's Arterial Preservation Program, innovative intersections and access management techniques were evaluated where applicable during the recommendations development of this study. Innovative intersections and access management inherently provide safety benefits by removing and separating conflict points that may exist in traditional intersection designs.

The most common method for determining the potential safety benefits of a roadway improvement is the calculation of expected crash reduction. This is done using crash reduction percentages from the Federal Highway Administration's (FHWA) Crash Modification Factors (CMF) Clearinghouse website, related safety research, and Virginia crash rate summaries and models. A CMF is an indicator of how crash occurrence will change as a result of a project based on evidence from similar improvements. A CMF less than 1.0 indicates a treatment that has a potential to reduce crashes. For example, a treatment with a CMF of 0.86 indicates that there is an expected 14 percent reduction in total estimated crash frequency. Table 4 displays fatal and injury crash CMFs used by VDOT for typical innovative intersections and access management treatments. Those in bold have been recommended at one or more areas along the corridor as part of this study.

Table 4. Crash Modification Factors

Improvement Type / Features	Fatal + Injury CMF							
Intersections								
Roundabout: Convert signal to roundabout	0.40							
Roundabout: Convert stop/yield control to roundabout	0.20							
Access Management: Close median opening (allow right-in right-out only)	0.40							
Two-way Stop Control to Restricted Crossing U-Turn	0.65							
Signal Control to Signalized Restricted Crossing U-Turn	0.80							
Signal Control to Continuous Green T Signal	0.85							
Stop Control to Continous Green T	0.85							
Displaced Left Turn	0.80							
Median U-Turn	0.70							
Interchanges								
Non-Freeway Segment: Convert Diamond to Diverging Diamond Interchange	0.30							
Non-Freeway Segment: Convert Diamond to Single Point Urban Interchange	0.60							
Segments								
Access Management: Reduce Driveway Density (eliminate/close)	0.70							
Access Management: Provide Median (allow right-in right-out only)	0.40							

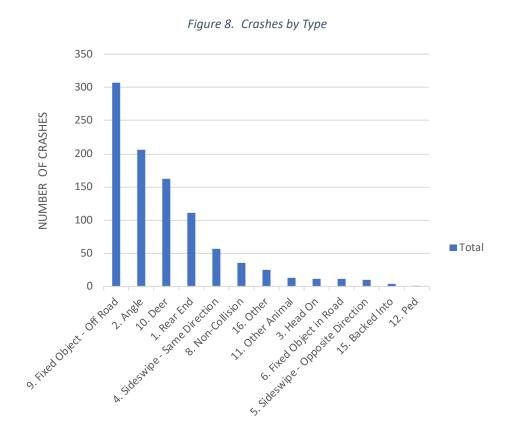


Figure 9. Crashes by Severity

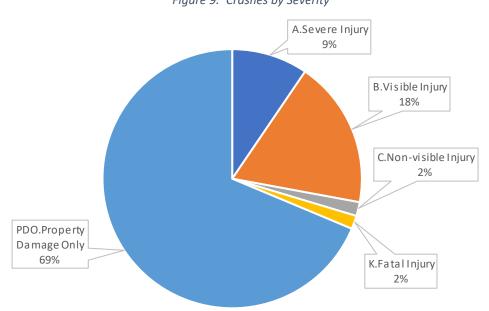


Figure 10. Corridor Crash Rates

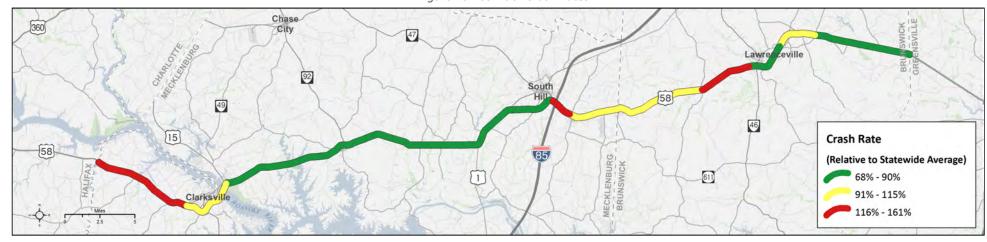


Figure 11. Corridor Crash Density

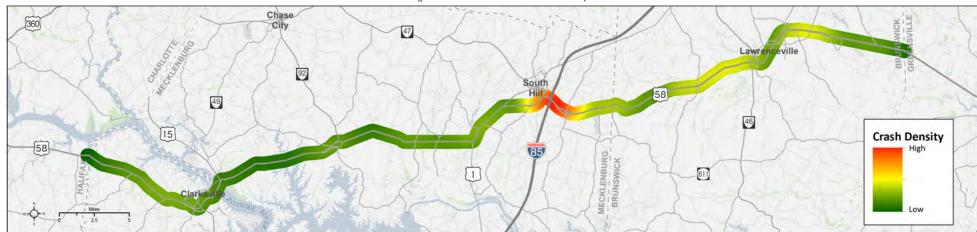


Figure 12. Corridor Cash Severity



2.5 Existing Traffic Volumes
Existing peak hour traffic volumes were developed using turn movement counts collected on May 16th, 2018 at the intersections listed below.

• US 58 / Business US 58 (Virginia Avenue) – Town of Clarksville

(9%) 14%

(0%) 2%

- US 58 / US 15 North Town of Clarksville
- US 58 / VA 92 (Washington Street) Mecklenburg County
- US 58 / VA 4 (Buggs Island Road) Mecklenburg County
- US 58 / US 1 (Big Fork) Mecklenburg County
- US 58 / VA 780 (Theater Road) Town of South Hill
- US 58 / VA 641 (Bright Leaf Road) Brunswick County

A full list of 2018 intersection volumes by AM and PM peak hour is found in Appendix D. The AM and PM peak hours are the times with the highest traffic volumes in the study area. The AM peak hour for analysis is 7:15 to 8:15. The PM peak hour for analysis is 4:45 to 5:45.

2.6 Existing Traffic Operations

The peak hour intersection turning movement counts developed in the previous section were analyzed in Synchro using the Highway Capacity Manual (HCM) module for both the AM and PM peak hours. Level of Service (LOS) is a qualitative measure used to relate the quality of traffic operations using letters A through F, where A represents free flow conditions and F represents extreme congestion. The operational analysis results for the study intersections are presented in Table 5. As shown in the table, all study intersections operate at LOS A for both peak hours, however congestion and delay increase as vehicles approach the Town of South Hill. Appendix E contains more detailed results of intersection operations for each intersection analyzed along the corridor.

Existing heavy vehicle percentages vary throughout the corridor. Heavy vehicle percentages are highest east of VA 92 (Washington Street). Further information about heavy vehicle percentages and volumes along specific segments of the study corridor is presented in Figure 13.

Figure 14 and Figure 15 represent the Travel Time Ratio (TTR) across the corridor, where TTR is defined as the ratio of commuting travel time to free-flow travel time. For example, a TTR of 1.10 indicates that the peak-period travel time is 10% greater than free-flow travel time.

Table 5. Existing Level of Service

2018 Existing Conditions										
Intersection	AM LOS	PM LOS								
Virginia Avenue & Route 58	А	А								
VA 92 & Route 58	А	А								
Kingdom Hall/US 1 & Route 58	А	А								
Theater Road & Route 58	А	А								
Main St(LaCrosse) & Route 58	А	В								
VA 641 (BrightLeaf Rd) & Route 58	A	A								

Heavy Vehicles Chase Westbound Lawre Eastbound 11% (5%) AM Peak XX 92 17% (9%) PM Peak 58 20% (7%) 49 15% (11%) Study Segments 46 3% (1%) (17%) 28% [15] 16% (6%) (4%) 10% (3%) 11% (3%) 8% (6%) 13%

Figure 13. Heavy Vehicle Percentages



Figure 15. PM Travel Time Ratio



CHAPTER 3: FUTURE CONDITIONS

3.1 Development of Growth Rates

Traffic volumes along the US 58 Corridor are anticipated to continue growing. Both Brunswick and Mecklenburg Counties note the US 58 corridor as appropriate for industrial development and commercial development along the roadway in their comprehensive plans. Future development, including proposed industrial parks near the Town of La Crosse, increased commercial development near the I-85 interchange, and the continued development of facilities such as the Microsoft Data Center, will contribute to traffic growth.

In addition to local growth, US 58 is the second busiest east-west corridor that connects the Port of Virginia to critical markets and that commercial growth is anticipated to continue. Updated traffic growth rates for the US 58 corridor were developed collaboratively using previous studies, historic traffic counts, the statewide travel demand model, and stakeholder input. The following sections outline the steps taken to develop the future 2040 traffic volumes.

3.1.1 Historical Average Annual Traffic Volumes and Travel Patterns

Historical average annual traffic volumes help establish a trend along the corridor and highlight segments where traffic volume may increase. The study team used VDOT historic traffic counts for fifteen segments in the corridor. For the historic data, VDOT collects traffic counts from sensors in average daily traffic (ADT) volume. Table 5 outlines these historic traffic volumes from 2010 to 2018.

3.1.2 Socio-Economic Data

This corridor plan derived estimated changes in population, households, and employment for the study area from the Statewide Travel Demand Model. Employment and population estimates are for the traffic analysis zones (TAZs) along the study corridor as shown in Figure 16. Table 6 summarizes the 2015 and 2040 estimates for population, household, and employment data from the Statewide Travel Demand Model for Brunswick and Mecklenburg Counties.

The socio-economic data from the Statewide Travel Demand Model shows an anticipated overall percent change for population, households, and employment in the study corridor TAZs. The corridor is anticipated to see modest growth with a 6% increase in population in Brunswick County and a 4% increase in population in Mecklenburg County. Employment along the corridor is anticipated to grow at a quicker pace with a 20% increase in employment for Brunswick County and a 9% increase in employment for Mecklenburg County.

3.1.3 Annualized Background Growth Rate

A one percent non-compounded annual background growth rate was developed using the historic traffic counts, statewide traffic model, existing documentation, and coordination with VDOT and the local communities. This background growth rate represents the expected increase in traffic volumes that travel through the entire US 58 study area and do not have an origin or destination along the route

Table 6. Historical Average Daily Traffic

			Histori	ical ADT						
From	То	2010	2011	2012	2013	2014	2015	2016	2017	2018
East Brunswick County Line	Old Stage Rd	6,900	9,200	9,100	8,900	9,100	9,700	9,300	9,400	9,200
Old Stage Rd	US 58 BUS/Lawrenceville Plank Rd	12,000	11,000	11,000	10,000	10,000	11,000	9,400	9,500	9,300
US 58 BUS/Lawrenceville Plank Rd	Cattail Rd	9,700	9,300	9,200	8,700	8,800	9,500	8,500	8,600	8,400
Cattail Rd	Grandy Rd	8,600	8,200	8,100	8,200	8,400	9,000	8,900	9,000	8,800
Grandy Rd	Mecklenburg County Line	9,800	9,400	9,300	9,100	9,300	10,000	10,000	10,000	10,000
Mecklenburg County Line	Country Club Rd	11,000	10,000	10,000	11,000	11,000	12,000	11,000	11,000	11,000
Country Club Rd	Country Lane	14,000	16,500	17,000	17,500	17,500	16,000	21,000	23,500	25,000
Country Lane	Theater Rd	6,400	6,100	6,000	6,100	6,200	6,700	7,000	7,200	7,100
Theater Rd	US 1	9,000	8,800	8,600	8,900	9,000	9,500	10,000	11,000	11,000
US 1	Buggs Island Rd	6,200	6,100	5,900	5,900	6,100	6,900	7,400	7,700	7,700
Buggs Island Rd	VA 92	5,800	6,000	5,600	5,500	5,500	5,900	6,500	6,800	6,700
VA 92	US 15	4,600	4,600	4,400	4,600	4,600	5,200	5,600	5,600	5,500
US 15	Virginia Ave	5,100	5,000	4,900	4,800	4,800	4,900	5,200	5,300	5,100
Virginia Ave	West Mecklenburg County Line	6,500	6,000	6,500	6,500	6,500	6,000	6,500	6,900	6,900

within the study area. The trip generation for the study area (discussed in the following section) and this background growth rate will be added to the existing traffic volumes to develop the future 2040 traffic volumes.

3.2 Projected Future Growth (2040) and Traffic Volumes

3.2.1 Future Land Use and Approved Development

Future land use was based on the socio-economic data in the travel demand model and stakeholder input. The study team looked at the projected population, household, and employment growth in the statewide travel demand model between 2015 and 2040 in TAZs within the study corridor. Figure 16 shows the TAZ growth along the corridor. Stakeholders reviewed these findings to assess the accuracy and provided feedback to the study team if adjustments to the assumed growth in certain TAZs were needed. These adjusted socio-economic datasets were used to estimate future traffic volumes in the study corridor and develop future traffic volumes at key intersections along the corridor.

Table 7. Employment and Population Growth Estimates

Jurisdiction		2015			2040		% Change (2015 - 2040)			
(TAZs)	Population	Households	Employment	Population	Households	Employment	Population	Households	Employment	
Brunswick County	10,621	3,641	4,669	11,278	3,848	5,584	6.2%	5.7%	19.6%	
Mecklenburg County	19,679	8,396	12,612	20,431	8,567	13,756	3.8%	2.0%	9.1%	
Total	30,300	12,037	17,281	31,709	12,415	19,340	4.7%	3.1%	11.9%	

3.2.2 Trip Generation and Distribution

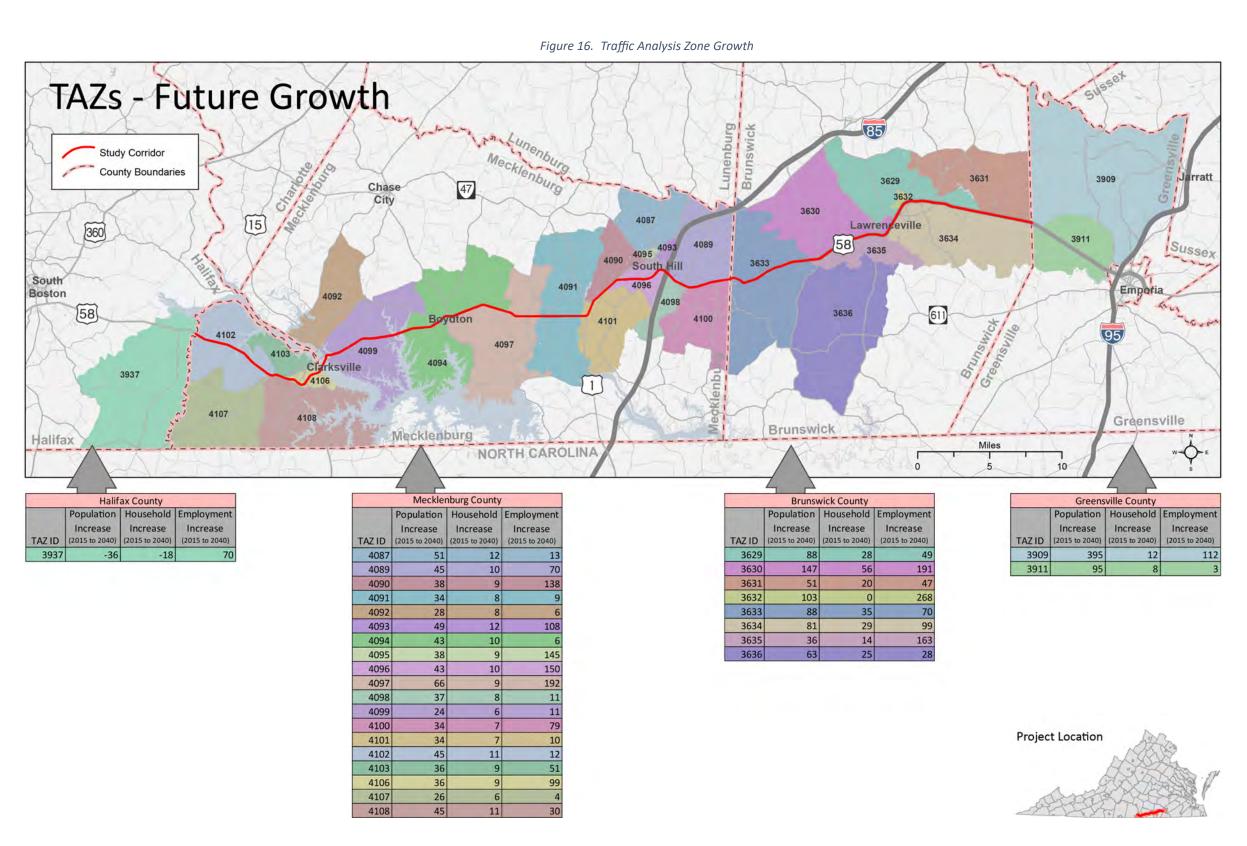
The study team evaluated the TAZs along the study corridor that have a direct effect on the turning movement counts used for the existing and future analyses. Traffic was then distributed at the study intersections based on the existing turning movement counts. With consideration for location, potential growth areas, and infrastructure off US 58, engineering judgement was used to make reasonable adjustments to the trip distribution. The future trip generation traffic volumes were added to the calculated background growth for the corridor and then used in the year 2040 analyses. The future turning movement volumes, trip generation, and background growth are outlined in Appendix D.

3.2.3 Future (2040) Traffic Volumes

Traffic volumes for the year 2040 were developed based on the trip generation discussed in the previous section and the background growth of one percent for the through traffic along the US 58 corridor. The projected 2040 volumes at various points within the study area are listed in Figure 16.

Table 8. Future Traffic Voumes

Future Traffic Volumes									
From	То	2018	2040						
East Brunswick County Line	Old Stage Rd	9,400	11,500						
Old Stage Rd	US 58 BUS/Lawrenceville Plank Rd	9,500	11,600						
US 58 BUS/Lawrenceville Plank Rd	Cattail Rd	8,600	10,500						
Cattail Rd	Grandy Rd	9,000	11,000						
Grandy Rd	Mecklenburg County Line	10,000	12,200						
Mecklenburg County Line	Country Club Rd	11,000	13,400						
Country Club Rd	Country Lane	23,500	28,700						
Country Lane	Theater Rd	7,200	8,800						
Theater Rd	US 1	11,000	13,400						
US 1	Buggs Island Rd	7,700	9,400						
Buggs Island Rd	VA 92	6,800	8,300						
VA 92	US 15	5,600	6,800						
US 15	Virginia Ave	5,300	6,500						
Virginia Ave	West Mecklenburg County Line	6,900	8,400						



CHAPTER 4: FUTURE (2040) TRAFFIC CONDITIONS

4.1 Future Traffic Operations

The 2040 future year operational analyses for the US 58 study intersections were performed using Synchro in accordance with VDOT's Traffic Operations and Safety Manual (TOSAM). Additional analysis was conducted in the Town of South Hill which included recommendations for the I-85 interchange. A summary of the additional analysis in the Town of South Hill is included in Appendix F. Although it is not known when the full build-out of the future land use will occur, the operational analysis for the 2040 scenarios includes the future traffic volumes for the full build-out of development to maximize the project life span for the recommended improvements. Two future traffic condition scenarios were analyzed. First, the no-build scenario assumes that US 58 will remain as is. Second, the build scenario assumes improvements will be made along US 58 as described further in Chapter 5. Tables 8 through 13 compare the analysis results of the existing, future no-build, and build conditions.

4.2 Future No-Build Traffic Operations and Deficiencies

Future traffic volumes, along with the background growth for through-vehicles, would have minimal impacts on most of the corridor based on the 2040 No-Build scenario. However, the Town of South Hill and La Crosse will experience delays up to LOS C in the AM and PM peak hours. Conventional signalized intersections do not have enough capacity to operate efficiently with extremely large traffic volumes and at unsignalized intersections, the through-movements along US 58 would not allow large enough gaps in traffic for turning movements to occur. Crashes would increase due to queue lengths extending into mainline traffic and the increases in stop-and-go traffic due to more congestion.

4.3 Results of Operational Analyses for Recommended Improvements

Chapter 5 details the recommended improvements, operations, and safety benefits of the recommendations. Although all the study intersections operated well in the future, recommendations were developed that focused focused on improving the safety of these intersections. The analysis was conducted to ensure that both safety and capacity would be satisfactory.

Recommendations consist mainly of innovative intersections concepts. Some of the recommendations include two or three intersections that function together as one system. Synchro does not currently have a method to analyze innovative intersections; however, Chapter 23 of the Highway Capacity Manual outlines a methodology for calculating delays and LOS by using travel time and the appropriate delay(s) through the innovative intersections. The HCM method provides a better way of comparing innovative intersections with the traditional intersection configurations that occupy the corridor today. All recommended improvements maintain an acceptable level of service of LOS C or better.

Table 9. Future Traffic Operations: Virginia Avenue and Route 58

Intersection	Scenario	Overall Delay	Delay per Lane Group by Approach (sec/veh) (Level of Service)											
		(LOS)		astboun	d	Westbound			Northbound			Southbound		
			LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
	AM Peak Hour													
	2010	2.7	8.1	0.0	N/A	0.0	0.0	0.0				13.3	N/A	9.4
	2018 Existing	2.7	Α	А	N/A	Α	А	А		N/A		В] N/A	Α
		Α		2.4 (A)			0.0 (A)						10.3(B)	
	2040 N	2.5	8.4	0.0	N/A	0.0	0.0	0.0				14.6	N/A	9.7
	2040 No Build	2.5	Α	Α	IN/A	Α	Α	Α	N/A		В	IN/A	Α	
		Α		2.2 (A)			0.0 (A)				10.9 (B)			
	2040 Build	4.2	8.4	0.0	N/A	0.0	0.0	0.0	N/A		12.8	N/A	9.7	
			Α	Α		Α	Α	Α			В	Α		
Virginia Avenue		Α	8.4 (A)		0.0 (A)					10.4 (B)				
& Route 58	PM Peak Hour													
	2040	4.3	8.3	0.0	N/A	0.0	0.0	0.0				14.3	N/A	9.9
	2018 Existing	4.5	Α	А	IN/A	Α	А	А	N/A	В	IN/A	В		
		А		3.9 (A)			0.0 (A)				10.8 (B)			
	2040 Na	3.9	8.6	0.0	0.0	0.0	0.0	0.0				15.5	N/A	10.4
	2040 No Build		Α	А	Α	Α	А	Α	N/A		С	NA	В	
		А		3.6 (A)			0.0 (A)						11.5 (B)	
	2040	5.2	8.8	0.0	N/A	0.0	0.0	0.0		14.5	N/A	10.6		
	Build	J.L	Α	А	.,,,	Α	А	А		N/A		В	.,,,,	В
	Dana	А		8.8 (A)			0.0 (A)				11.4 (B)			

Table 10. Future Traffic Operations: VA 92 and Route 58

Intersection	Scenario	Overall Delay	Delay per Lane Group by Approach (sec/veh) (Level of Service)											
		(LOS)	Eastbound			Westbound			Northbound			Southbound		
			LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
	AM Peak Hour													
	2018	3.3	7.9	0.0	0.0	7.7	0.0	0.0	11.1	11.1	11.1	11.8	11.8	11.8
	Existing		Α	А	Α	Α	Α	Α	В	В	В	В	В	В
		А	1.4 (A)		0.3 (A)			11.1 (B)			11.8 (B)			
	2040 No Build	3.0	8.1	0.0	0.0	7.9	0.0	0.0	11.8	11.8	11.8	12.8	12.8	12.8
			Α	Α	Α	Α	Α	Α	В	В	В	В	В	В
		Α	1.1 (A)			0.4 (A)			11.8 (B)			12.8 (B)		
	2040	3.2	8.0	0.0	0.0	12.0	0.0	0.0	N/A	N/A	9.2	13.6	21.3	9.3
	Build		Α	А	Α	Α	А	А			Α	В	С	А
VA 92 & Route		А	1.1 (A)			0.5 (A)				9.2 (A)		14.6 (B)		
58	PM Peak Hour													
	2018	4.3	8.0	0.0	0.0	7.5	0.0	0.0	11.0	11.0	11.0	11.4	11.4	11.4
	Existing		Α	А	А	Α	А	А	В	В	В	В	В	В
		Α		1.0 (A)			0.1 (A)			11.0 (B)		11.4 (B)		
	2040 No Build	3.9	8.3	0.0	0.0	7.7	0.0	0.0	11.7	11.7	11.7	12.4	12.4	12.4
	Bulla		Α	Α	Α	Α	Α	А	В	В	В	В	В	В
		Α		0.8 (A)			0.1 (A)			11.7 (B)		12.4 (B)		
	2040 Build	3.6	8.8	0.0	0.0	12.0	0.0	0.0	N/A	N/A	9.0	13.0	22.3	10.3
	Bulla		Α	А	Α	Α	Α	А			Α	В	С	В
		А		0.9 (A)			0.2 (A)			9.0 (A)			12.8 (B)	

Table 11. Future Traffic Operations: Kingdom Hall/US 1 and Route 58

Intersection	Scenario	Overall Delay	Delay per Lane Group by Approach (sec/veh) (Level of Service)											
		(LOS)	Eastbound			Westbound			Northbound			Southbound		
			LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
	AM Peak Hour													
	2010	2.7	0.0	0.0	0.0	8.3	0.0	0.0	11.1	11.1	11.1	0.0	0.0	0.0
	2018 Existing	2.7	Α	Α	Α	Α	Α	Α	В	В	В	Α	Α	Α
	LAISTING	А	0.0 (A)			1.3 (A)			11.1 (B)			0.0 (A)		
		3.1	0.0	0.0	0.0	8.6	0.0	0.0	11.8	11.8	11.8	0.0	0.0	0.0
	2040 No Build	5.1	Α	А	Α	Α	Α	Α	В	В	В	А	Α	Α
	Dana	А	0.0 (A)			1.2 (A)			11.8 (B)			0.0 (A)		
		3.0	N/A	0.0	0.0	8.3	0.0	0.0	13.4	N/A	10.8	N/A	N/A	0.0
	2040 Build	3.0	IN/A	А	А	Α	Α	Α	В	IN/A	В	IN/A	IN/A	Α
Kingdom Hall/US	Bana	А		0.0 (A)			1.1 (A)			11.3 (B)			0.0 (A)	
1 & Route 58	PM Peak Hour													
		2.2	0.0	0.0	0.0	8.6	0.0	0.0	11.2	11.2	11.2	0.0	0.0	0.0
	2018 Existing	2.2	А	А	А	А	Α	Α	В	В	В	А	А	Α
	LXISTING	А		0.0 (A)			0.0 (A)			11.2 (A)		0.0 (A)		
		2.6	0.0	0.0	0.0	8.9	0.0	0.0	11.7	11.7	11.7	0.0	0.0	0.0
	2040 No Build	2.6	А	Α	А	Α	Α	Α	В	В	В	Α	Α	Α
	Dalla	А		0.0 (A)			2.1 (A)		11.7 (B)			0.0 (A)		
		2.6	NI/A	0.0	0.0	8.8	0.0	0.0	16.5	NI/A	10.6	NI/A	N/A	0.0
	2040 Build	2.6	N/A	А	А	Α	А	А	С	N/A	В	N/A		Α
	Bullu	А		0.0 (A)			2.1 (A)			11.4 (B)			0.0 (A)	

Table 12. Future Traffic Operations: Theater Road and Route 58

Intersection Scenario Overall Delay per Lane Group by Approach (sec/veh) (Level of Service) Delay (LOS) Eastbound Westbound Northbound Southbound LT TH RT LT TH RT LT TH RT LT TH RT AM Peak Hour 9.8 0.0 0.0 7.8 0.0 0.0 19.2 19.2 19.2 | 24.1 | 24.1 24.1 3.4 2018 С С С С С Α Α Α Α Α С Existing 5.1 (A) 0.0 (A) 19.2 (C) 24.1 (C) Α 10.3 0.0 0.0 8.2 0.0 0.0 22.9 22.9 22.9 28.4 28.4 28.4 3.0 2040 No В В В Α Α Α C C С D D D Build Α 4.2 (B) 0.0 (A) 22.9 (C) 28.4 (D) 0.0 22.9 22.9 22.9 | 28.4 | 28.4 | 28.4 10.3 0.0 0.0 8.2 0.0 3.0 2040 В Α Α Α С С С D D D Build Α 4.2 (B) 0.0 (A) 22.9 (C) 28.4 (D) Theater Road & Route 58 PM Peak Hour 0.0 15.9 | 15.9 | 18.2 18.2 18.2 8.4 0.0 0.0 8.3 0.0 15.9 3.0 2018 С Α С С С Α Α Α Α С С Existing Α 3.1 (A) 0.1 (A) 15.9 (C) 18.2 (C) 8.9 0.0 0.0 8.7 0.0 0.0 18.5 18.5 | 18.5 | 21.2 | 21.2 | 21.2 2.6 2040 No Α C C C C С C Α Α Α Α Α Build 2.7 (A) 21.2 (C) Α 0.1 (A) 18.5 (C) 8.7 18.5 18.5 | 18.5 | 21.2 | 21.2 | 21.2 8.9 0.0 0.0 0.0 0.0 2.6 2040 С Α Α Α С С С Α Build 2.7 (A) 0.1 (A) 18.5 (C) 21.2 (C)

Table 13. Future Traffic Operations: Main Street (LaCrosse) and Route 58

Intersection	Scenario	Overall Delay	Delay per Lane Group by Approach (sec/veh) (Level of Service)											
		(LOS)	Eastbound		Westbound			Northbound			Southbound			
			LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
	AM Peak Hour													
	2040	8.7	36.0	5.2	4.7	26.5	5.6	4.5	25.3	25.3	25.3	21.0	21.0	21.0
	2018 Existing	0.7	D	Α	Α	С	А	Α	С	С	С	С	С	С
		Α		6.2 (A)		5.8 (A)			25.3 (C)			21.0 (C)		
	2040 No Build	14.1	157.7	8.5	7.5	44.1	10.2	6.9	27.6	27.6	27.6	18.3	18.3	18.3
		14.1	F	Α	Α	D	В	Α	С	С	С	В	В	В
		В	12.9 (B)			10.6 (B)			27.6 (C)			18.3 (B)		
	2040 Build	6.6	59.8	4.8	0.3	26.1	5.8	0.0	16.1	16.1	16.1	30.5	25.0	13.7
		0.0	Α	Α	Α	В	Α	Α	В	В	В	С	С	В
Main St(LaCrosse) &	20.10	Α		5.4 (A)		6.1 (A)			16.1 (B)				16.7 (B)	
Route 58	PM Peak Hour													
	2040	12.5	64.0	7.3	6.5	28.8	7.9	6.6	26.0	26.0	26.0	18.9	18.9	18.9
	2018 Existing	12.5	Е	А	А	С	Α	Α	С	С	С	В	В	В
		В		11.5 (B)			8.1 (A)		26.0 (C)			18.9 (B)		
	2040 NI-	22.2	81.9	10.2	9.2	42.3	12.1	8.7	66.3	66.3	66.3	18.0	18.0	18.0
	2040 No Build		F	В	А	D	В	Α	E	E	Е	В	В	В
		С		14.1 (B)			12.5 (B)			66.3 (E)	•	18.0 (B)		
	2040	10.9	62.1	7.1	0.3	27.4	7.1	0.0	25.1	25.1	25.1	27.0	20.2	8.9
	2040 Build	10.5	F	А	А	В	А	Α	С	С	С	С	С	В
		В		8.1 (A)			7.3 (A)			25.1 (C)			12.6 (B)	

Table 14. Future Traffic Operations: VA 641 (Bright Leaf Road) and Route 58

Intersection	Scenario	Overall Delay		Delay per Lane Group by Approact (Level of Service)							ch (sec/veh)					
		(LOS)	Eastbound			Westbound			Northbound		Southbound		nd			
			LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
	AM Peak Hour															
		1.7	8.6	0.0	N/A	N/A	0.0	0.0				10.8	N/A	10.8		
	2018 Existing	1.7	Α	Α	IN/A	IN/A	Α	Α		N/A		В	IN/A	В		
		А	2.1 (A)		0.0 (A)						10.8 (B)					
	2040 N -	1.8	9.1	0.0	N/A	N/A	0.0	0.0				11.7	N/A	11.7		
	2040 No Build	1.0	Α	Α	IN/A	IN/A	Α	Α		N/A		В		В		
		Α		2.0 (A)			0.0 (A)					11.7 (B)				
	2040 Build	0.9	9.1	0.0	N/A	N/A	0.0	0.0			11.7	N/A	10.0			
			Α	А		N/A	A A	Α		N/A		В	N/A	В		
VA 641 (BrightLeaf Rd)		А	0.3 (A)			0.0 (A)						10.0 (B)				
& Route 58	PM Peak Hour															
	2010	1.4	8.8	0.0	N/A	N/A	0.0	0.0				11.2	N/A	11.2		
	2018 Existing	1.4	Α	А	N/A	N/A	Α	А		N/A		В	N/A	В		
		А		0.8 (A)			0.0 (A)					11.2 (B)				
	2040 No	1.6	9.4	0.0	N/A	N/A	0.0	0.0			12.5	N/A	12.5			
	Build	1.0	Α	А	IN/A	NA	А	А		N/A		В	19/7	В		
		Α		1.0 (A)			0.0 (A)					12.5 (B)				
	2040	1.6	9.2	0.0	N/A	N/A	0.0	0.0	N/A		12.6	N/A	10.7			
	2040 Build		Α	А		14//	Α	Α				В	IN/A	В		
		А		0.9 (A)			0.0 (A)						11.6 (B)			

CHAPTER 5: ALTERNATIVES AND RECOMMENDATIONS

5.1 US 58 Corridor Recommendations

Future traffic volumes show that the US 58 corridor needs improvements to maintain capacity and improve safety. The majority of these improvements are needed to maintain regional growth and improve roadway safety. Additional improvements such as crossover closings may be implemented immediately to increase safety through access management. Based on capacity analyses of current and future conditions and a review of current corridor infrastructure, a "toolbox" of improvements was developed for the US 58 study area. These include:

- Remove existing crossover (based on inadequate spacing/grade/etc.);
- Upgrade existing crossover to meet VDOT standards;
- Convert existing crossover to directional median to allow only certain movements;
- Install alternative intersection concepts; and
- Improve shoulder widths to meet VDOT requirements

Alternative intersections and access management techniques were evaluated during the development of recommendations. Below is a list of alternative intersection designs that are included in the VDOT Arterial Preservation Plan toolbox that were evaluated as potential recommendations. Some of the alternative designs were not suitable for certain locations due to the geometric constraints, concept's principles, associated costs, and/or Right-of-Way limitations. The concepts listed below were evaluated to screen individual concepts at every location to determine the most effective options for analysis and recommendation.

- Median U-turn Intersection (MUT)
- Restricted Crossing U-turn Intersection (RCUT)
- Continuous Green-T (CGT)
- Quadrant Roadway (QR)

Detailed information on each of these concepts is available on VDOT's Innovative Intersections website located at http://www.virginiadot.org/innovativeintersections/.

It is well documented that as the number of access points increase along a corridor, the running speed decreases and the number of crashes increase. Given that the study segments of US 58 are of vital importance to the state and region, it is important to ensure the safety and throughput capacity of the corridor.

Recommendations were developed using the crash evaluation and analysis of the future volumes from both planned and potential developments along the study corridor. Project stakeholders and the public were engaged throughout the project process to identify the most preferred recommendations. These recommendations are presented in Appendix A. Table 15 contains a suggested ranking of the recommendations based on crash history and the VDOT Potential for Safety Improvements (PSI) database. Recommendation locations are highlighted on corridor aerial photos, with the identification circle indicating the type of recommendation. A green circle indicates no recommendation, a red circle indicates a recommended crossover removal, a yellow circle indicates a minor improvement, and a blue circle indicates a major improvement. Recommendations are denoted with C# for crossovers and I# for intersections. The written recommendation description is available by finding the corresponding C# or I# in the right-hand information box. For complex recommendations, the description will refer to a

figure with a detailed project sketch. Cost estimates were developed using the VDOT Transportation and Mobility Planning Division (TMPD) Cost Estimate Spreadsheet tool and the figures include the range of costs in 2019 dollars for each recommendation.

It is intended that the recommendations presented in Appendix A will accommodate the full build-out of development identified in the future land use as well as the increased vehicular through-put on US 58. As part of this US 58 Arterial Preservation Plan, it is recommended that no additional traffic signals be installed other than those listed in the recommendations. As well, it is recommended that no additional crossovers be constructed within the US 58 median beyond the Preservation Plan recommendations.

Additional shoulder widths with safety edges, when applicable, are recommended to be constructed in areas that do not meet minimum design standards. All shoulders should be paved to the VDOT design standard of eight feet or better to accommodate disabled vehicles, vehicles entering and exiting residential and commercial driveways, and bicyclists. In areas where the existing grade does not support the minimum shoulder requirements, guardrail should be installed.

5.2 Possible Funding Sources

Implementation of the recommended improvements will require funding sources. The VDOT SMART SCALE Program is a process that invests in projects that meet the most critical transportation needs in the state. Projects are evaluated based on improvements in certain categories such as congestion and safety. At the corridor level, more specific strategies and operational improvements can be assessed in studies and implemented using a variety of funding sources, including Federal funding streams such as the Surface Transportation Program (STP), National Highway System (NHS) funds, the Congestion Mitigation and Air Quality Improvement (CMAQ) Program, Revenue Sharing, Highway Safety Improvement Program (HSIP), as well as through state or local funding or other discretionary funding sources. For larger projects, particularly capacity-adding projects, demand management, and operational strategies should also be analyzed for incorporation into the project as part of the project development process. The complex recommendations presented in Appendix A, Figures 5, 12, 13, 19, 23, 24, 25, 26, 28, 35, and 38 include improvement types that correspond with the categories required for specific funding sources.

Table 15. Suggested Priority for US 58 Recommendations

Recommendation Figure	Intersecting US 58 Roadway	Jurisdiction	High Cost (\$ Millions)	Total Crashes (2013 - 2018)	VTrans Needs Met	Economic Development Support	Congestion (Existing LOS)	Crash Rank	Congestion Rank	ED Rank	Total Score	Cost/Score	Rank
24	Country Ln	Town of South Hill	3.1	19	2	Υ	С	27	35	31	30.45	9.82	1
26	Roundabout - Eastern Corporate Limits	Town of South Hill	6	20	2	Υ	А	28	1	31	27.35	4.56	2
26	Cycle/Peebles - Eastern Corp	Town of South Hill	2.3	43	2	Υ	В	33	34	31	32.15	13.98	3
28	N Main St	Town of La Crosse	1.1	16	1	N	А	26	1	1	8.5	7.73	4
32	Robinson Ferry Rd	Brunswick County	1	10	1	N	А	21	1	1	7	7.00	5
29	Regional Airport Rd	Mecklenburg County	1	7	1	N	А	18	1	1	6.1	6.10	6
35	Cattail Dr	Town of Lawrenceville	3.4	13	1	Υ	А	25	1	31	18.7	5.50	7
41	Freemans Crossing Rd	Brunswick County	1.3	10	1	N	А	21	1	1	7	5.38	8
5	Virginia Ave	Town of Clarksville	1.4	6	1	N	А	16	1	1	5.5	3.93	9
17	Baskerville Rd	Mecklenburg County	1.1	4	1	N	А	12	1	1	4.3	3.91	10
32	Evans Creek Rd	Brunswick County	2.2	12	1	N	А	23	1	1	7.6	3.45	11
10	Skipwith Rd	Town of Boydton	1.6	6	1	N	А	16	1	1	5.5	3.44	12
38	Bright Leaf Rd/Airport Dr	Brunswick County	2.3	12	1	N	А	23	1	1	7.6	3.30	13
33	Pleasant Grove Rd	Brunswick County	2.1	7	1	N	А	18	1	1	6.1	2.90	14
21	Goods Ferry Rd	Mecklenburg County	1.5	4	1	N	А	12	1	1	4.3	2.87	15
2	Cherry Hill Church Rd	Mecklenburg County	1	3	1	N	А	7	1	1	2.8	2.80	16
3	Clarksville Rd	Mecklenburg County	1	3	1	N	А	7	1	1	2.8	2.80	17
12	Washingston St	Town of Boydton	1.9	5	1	N	А	15	1	1	5.2	2.74	18
2	Buffalo Springs Rd	Mecklenburg County	1	2	1	N	А	5	1	1	2.2	2.20	19
Segment	Landfill Rd to Buggs Island Rd	Mecklenburg County	5	24	1	N	А	30	1	1	9.7	1.94	20
Segment	Union Woods Rd to Pleasant Grove Rd	Brunswick County	5.7	50	1	N	А	34	1	1	10.9	1.91	21
Segment	Evans Creek Rd to Grandy Rd	Brunswick County	7.5	64	1	N	А	35	1	1	11.2	1.49	22
Segment	Branch Rd to Gholson Rd	Brunswick County	6.1	20	1	N	А	28	1	1	9.1	1.49	23
Segment	Airport Dr to Old Stage Rd	Brunswick County	7.2	35	1	N	А	32	1	1	10.3	1.43	24
14	Hayes Mill Rd	Mecklenburg County	1.7	2	1	N	А	5	1	1	2.2	1.29	25
30	Dornia Ave	Brunswick County	3.8	4	1	N	А	12	1	1	4.3	1.13	26
25	I-85	Town of South Hill	28	30	2	Υ	А	31	1	31	28.25	1.01	27
2	Tabernacle Rd	Mecklenburg County	1	1	1	N	А	1	1	1	1	1.00	28
6	Shiney Rock Rd	Town of Clarksville	1	1	1	N	А	1	1	1	1	1.00	29
10	Jefferson St	Town of Boydton	3.1	3	1	N	Α	7	1	1	2.8	0.90	30
8	Tower Rd	Mecklenburg County	1.3	1	1	N	А	1	1	1	1	0.77	31
30	Main St	Brunswick County	3.8	3	1	N	Α	7	1	1	2.8	0.74	32
13	US 58 BUS	Town of Boydton	1.4	1	1	N	Α	1	1	1	1	0.71	33
19	US 1	Mecklenburg County	9.7	9	1	N	А	20	1	1	6.7	0.69	34
10	Mayfield Dr	Town of Boydton	4.6	3	1	N	А	7	1	1	2.8	0.61	35

Scores reflect weighting by SMART SCALE area type categories.

Although the score of Cycle/Peebles - Eastern Corp Limits South Hill is higher, the roundabout must occur first before pursuing this option. This has been reflected in the recommended priority.

APPENDICES

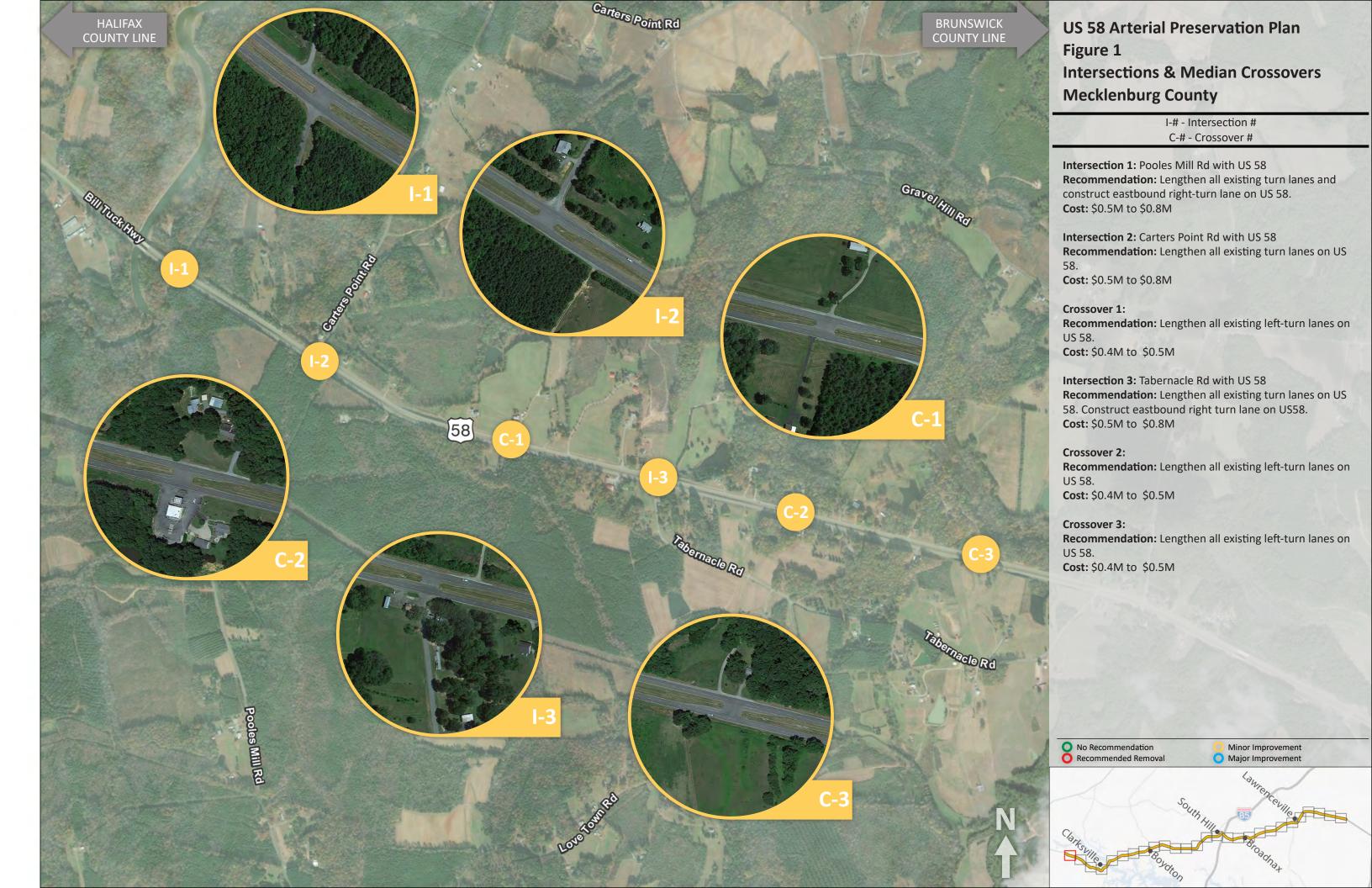
Appendix A: US 58 Arterial Preservation Plan Recommendations	A-1
Appendix B: Field Review	A-45
Appendix C: Infrastructure Inventory	A-57
Appendix D: US 58 Intersection Volumes	A-81
Appendix E: US 58 Intersection Operations	A-85
Appendix F: Town of South Hill Memorandum	A-89

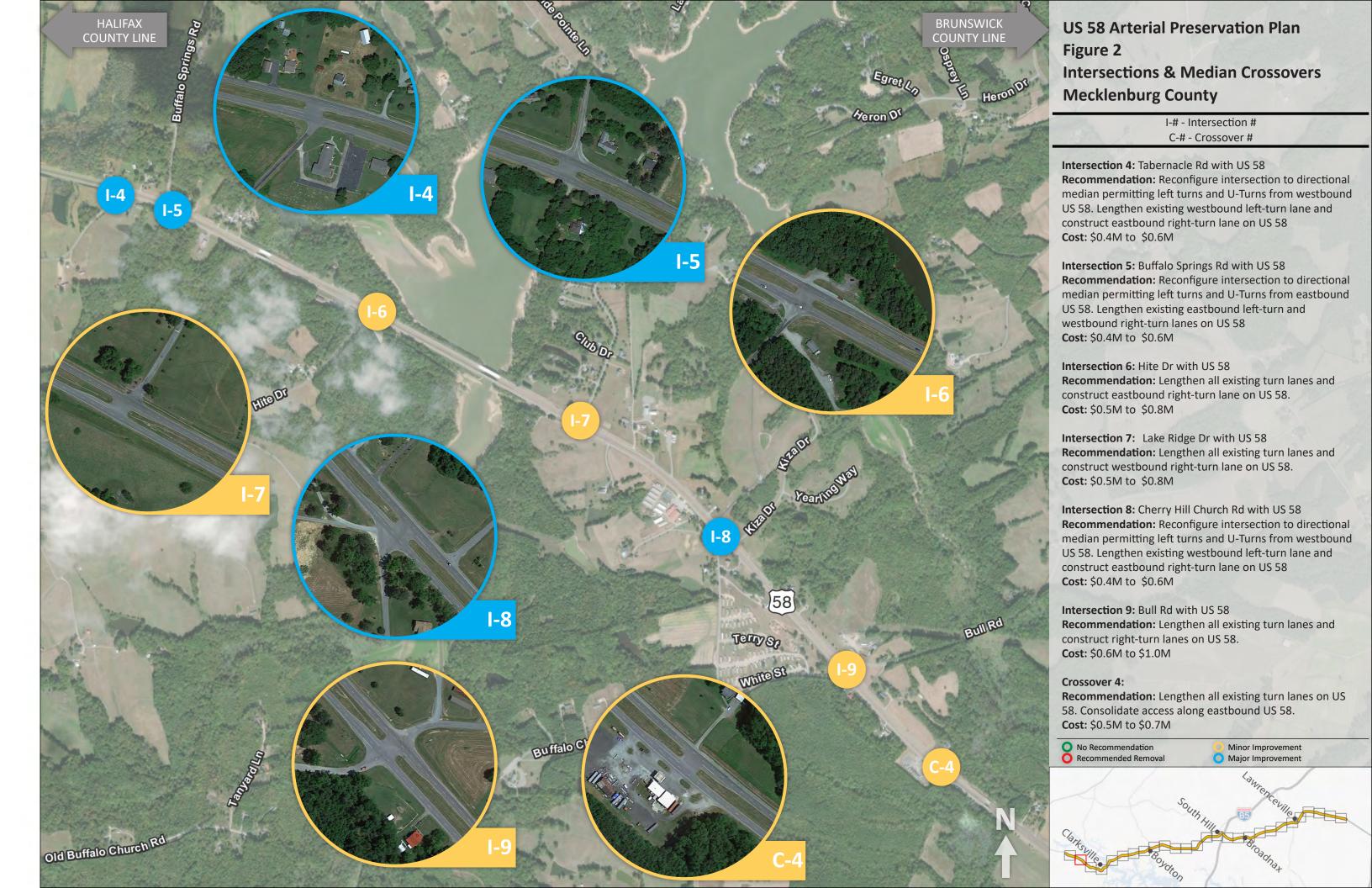
A-1			
-45			
-57			
-81			
-85			
-89			

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APPENDIX A: US 58 ARTERIAL PRESERVATION PLAN RECOMMENDATIONS

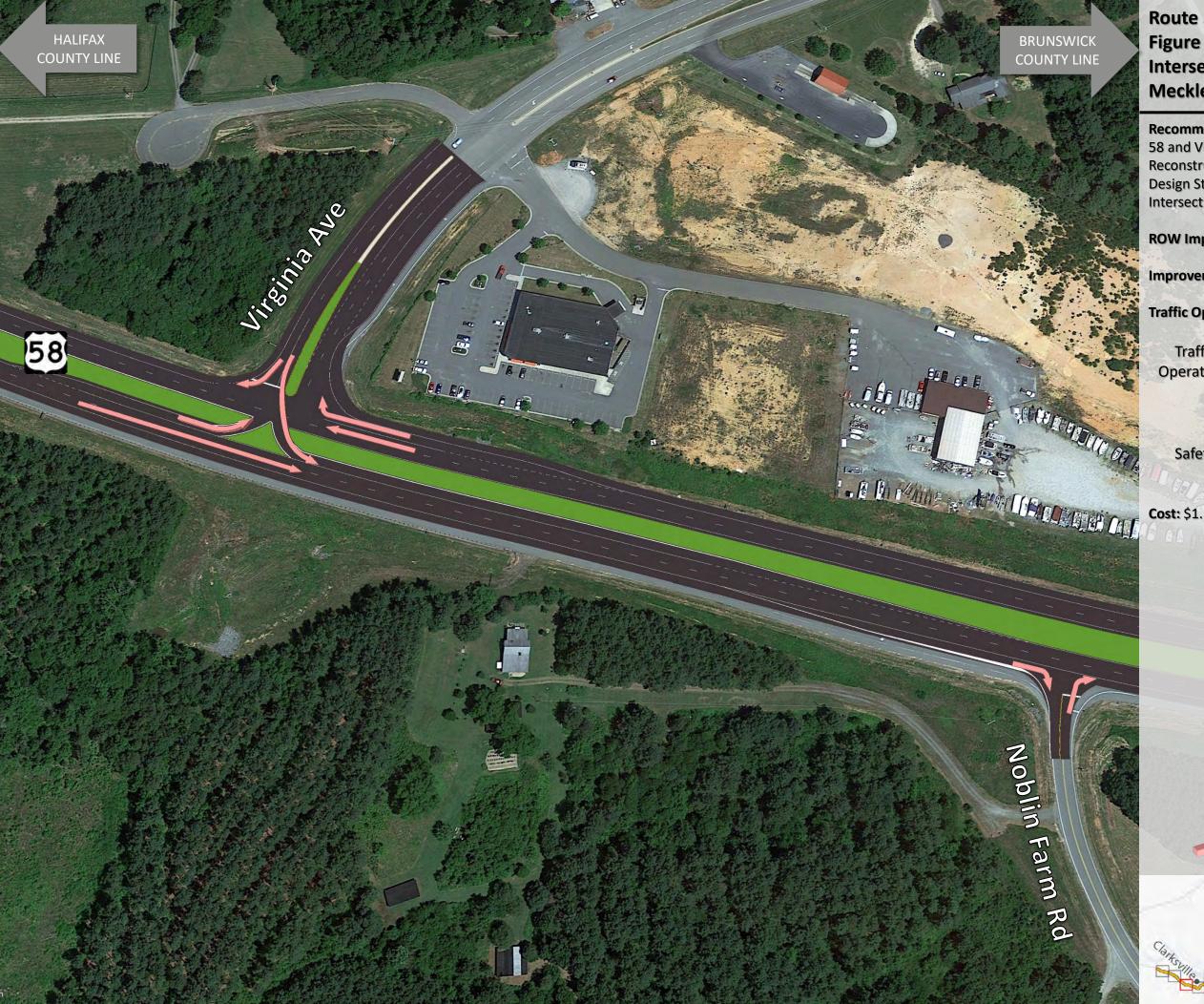
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Route 58 Arterial Management Plan Figure 5

Intersection #14: US 58 and Virginia Ave **Mecklenburg County**

Recommendation: Reconfigure main intersection of US 58 and Virginia Ave to Continuous Green-T (CGT). Reconstruct existing turn-lanes on US 58 to VDOT Design Standards. Improvements are required at Intersection 15, Noblin Farm Rd and US 58.

ROW Impacts: All improvements are within the ROW

Improvement Type: Safety, Travel Time Preservation

Traffic Operations & Safety:

Traffic Operations Reduced delay times for vehicles traveling eastbound on US 58 from Virginia Ave

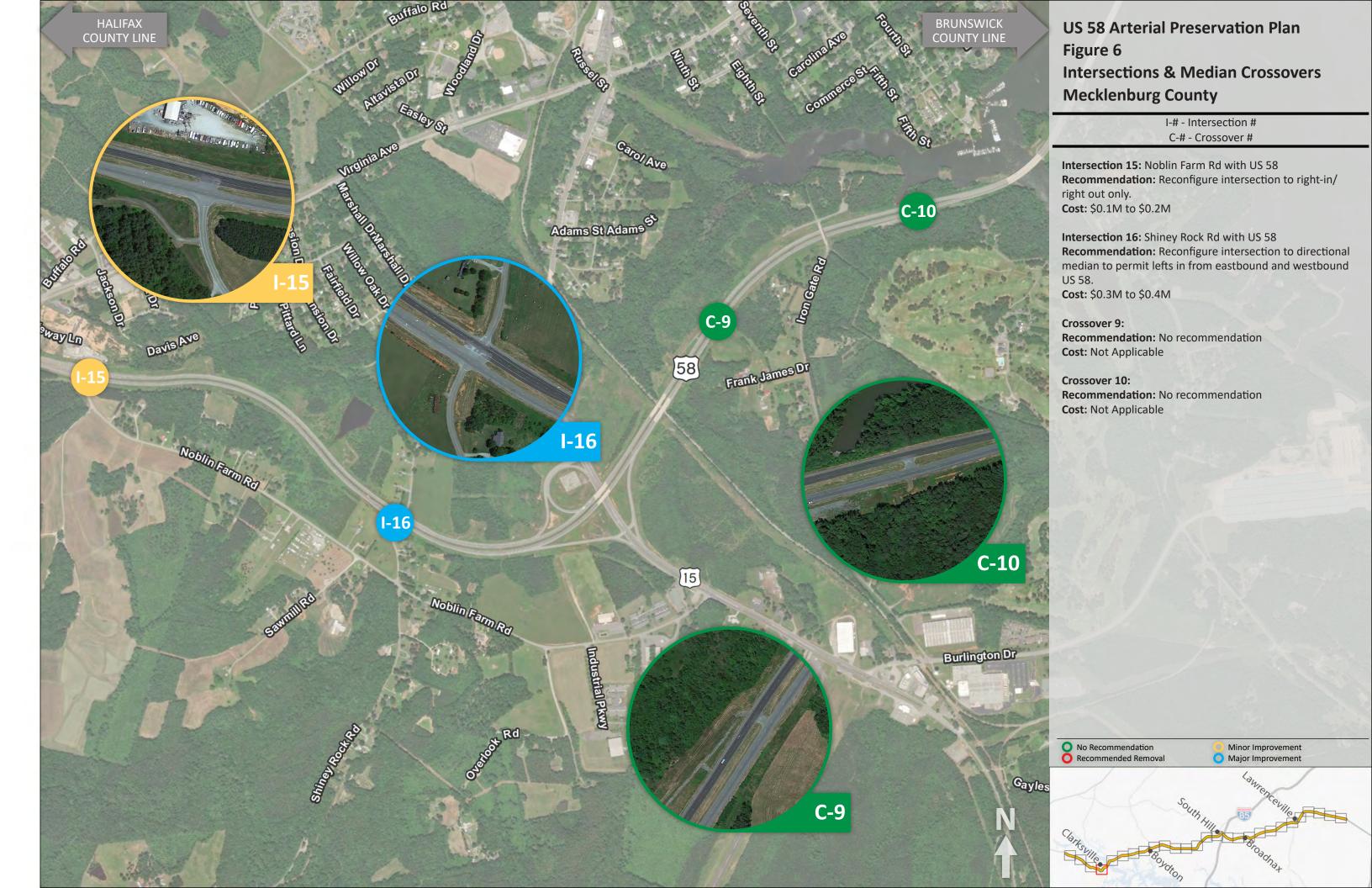
Safety

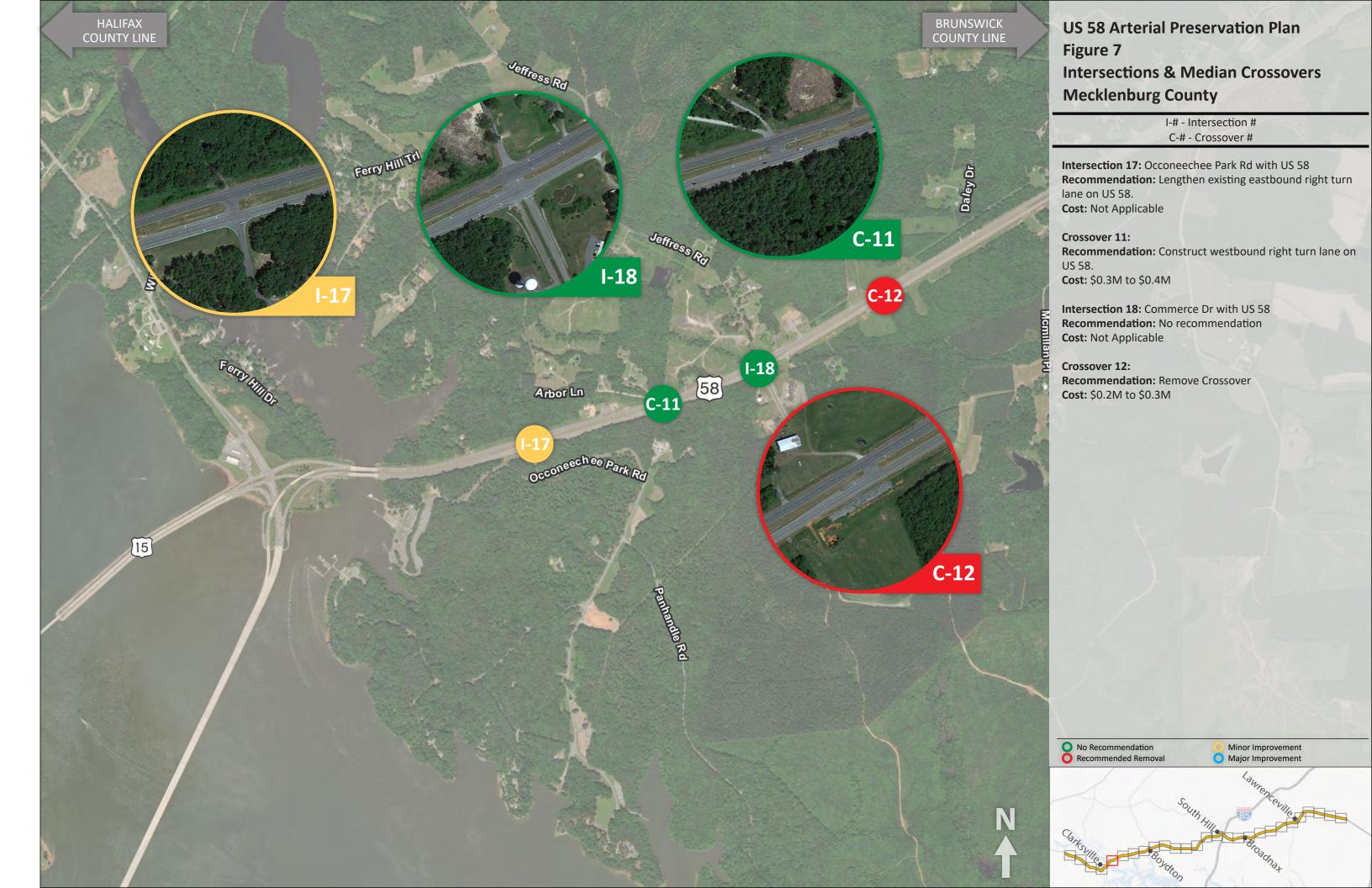
Reduced conflict points where vehicles cross paths. Reduced risk of angle crashes from Virginia Ave onto US 58

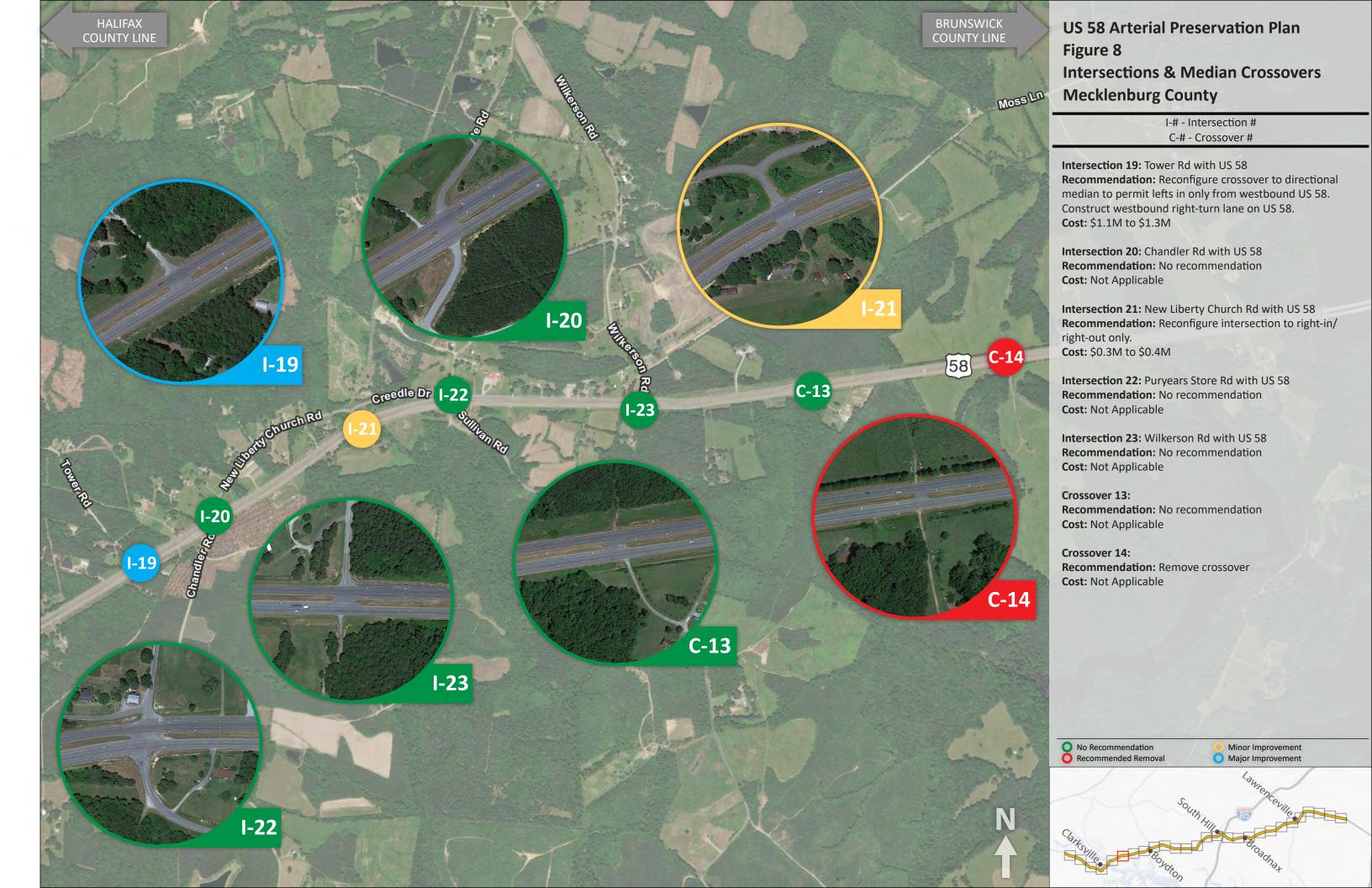
Cost: \$1.1M to \$1.4M

Standard Movements

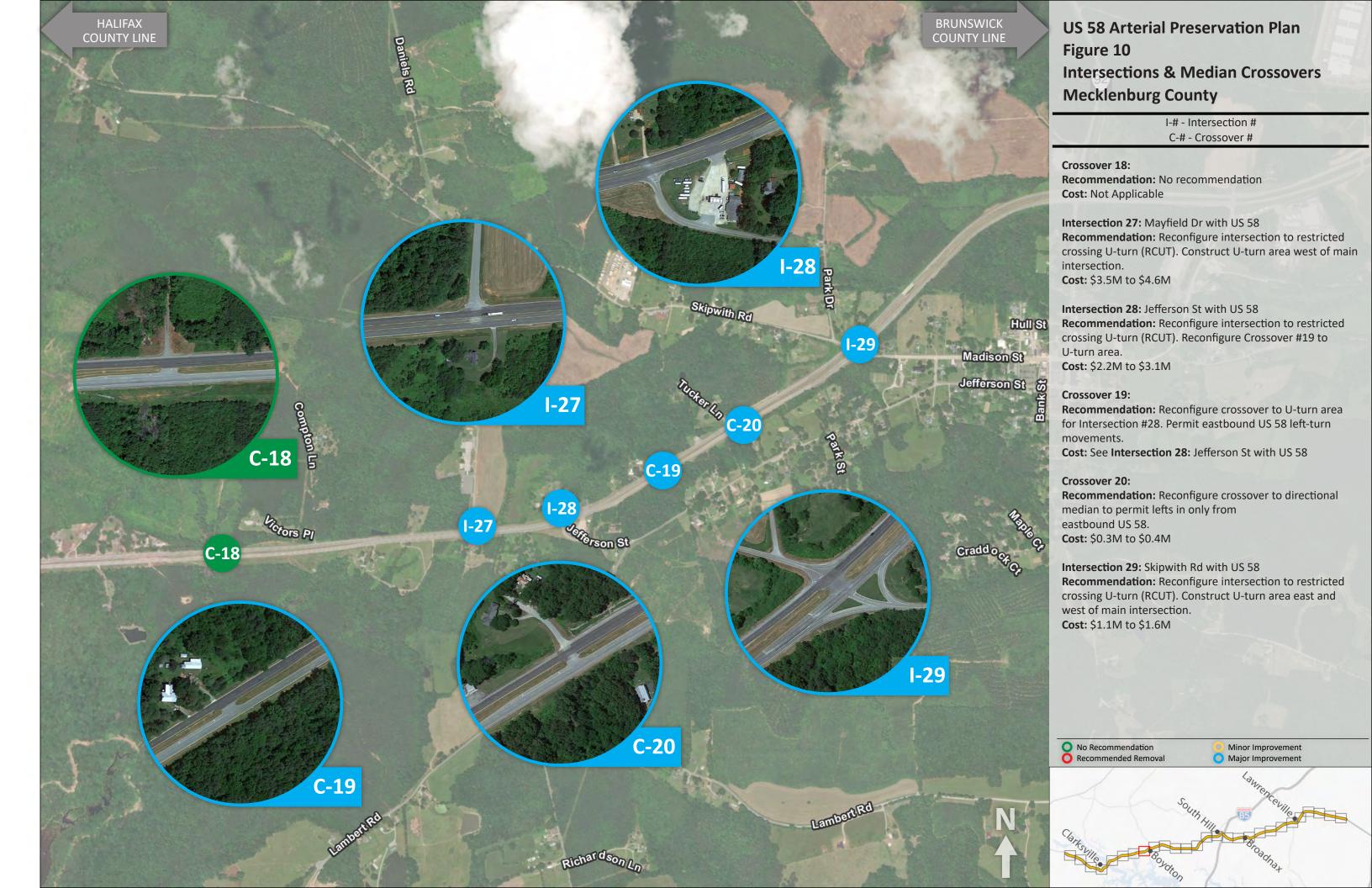


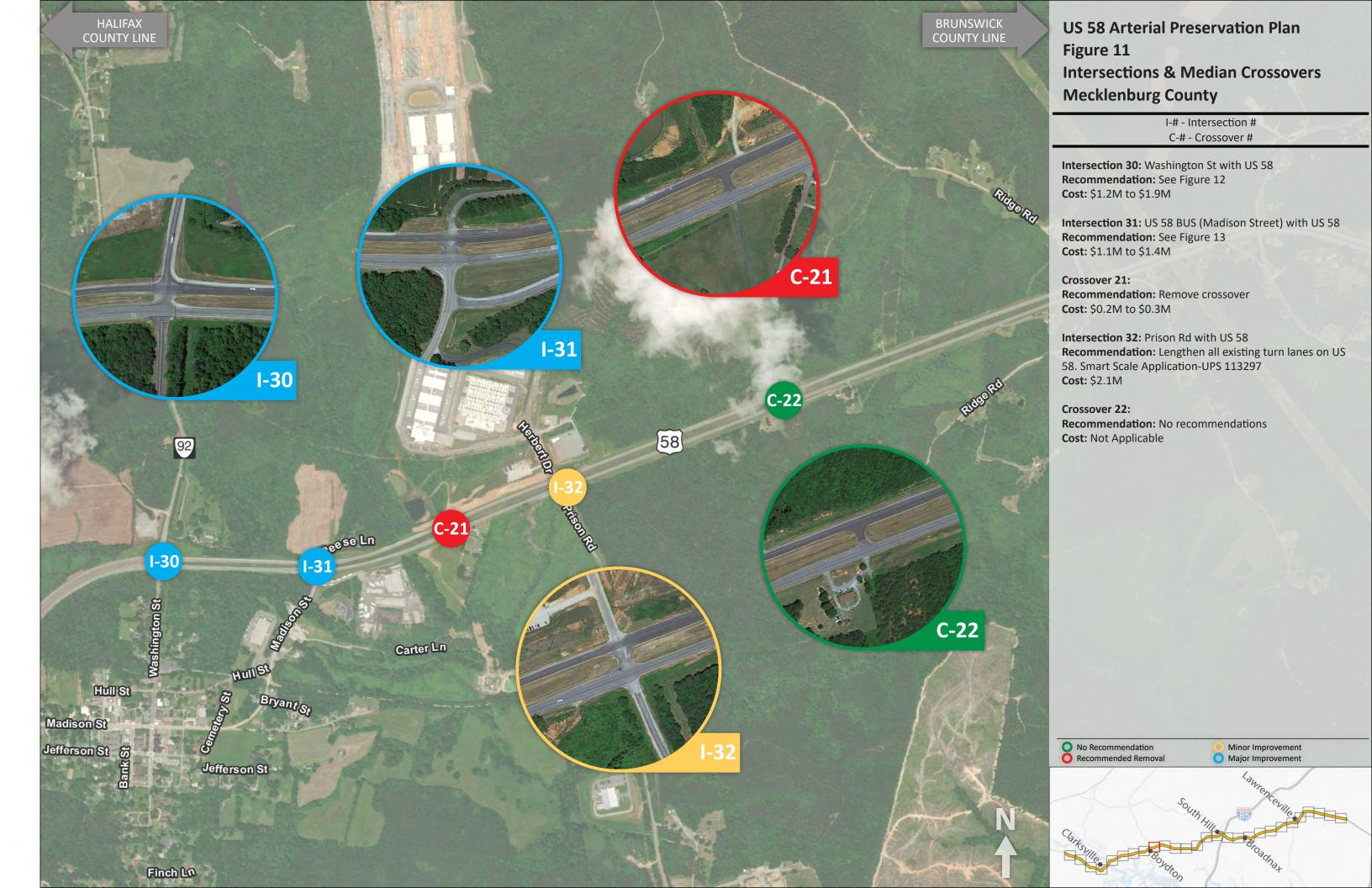














Route 58 Arterial Management Plan Figure 12

Intersection #30: US 58 and Washington St Mecklenburg County

Recommendation: Reconfigure intersection of US 58 and Washington St to Continuous Green-T (CGT). Construct U-turn area west of main intersection to permit southbound movements from Washington St to cross US 58. Extend eastbound right-turn lane to U-turn area and reconstruct existing turn-lanes on US 58 to VDOT Design Standards.

ROW Impacts: All improvements are within the ROW

Improvement Type: Safety, Travel Time Preservation

Traffic Operations & Safety:

Traffic Operations

Reduced delay times for vehicles traveling eastbound on US 58 from southbound Washington St

Safety

Reduced conflict points where vehicles cross paths. Reduced risk of angle crashes from Washington

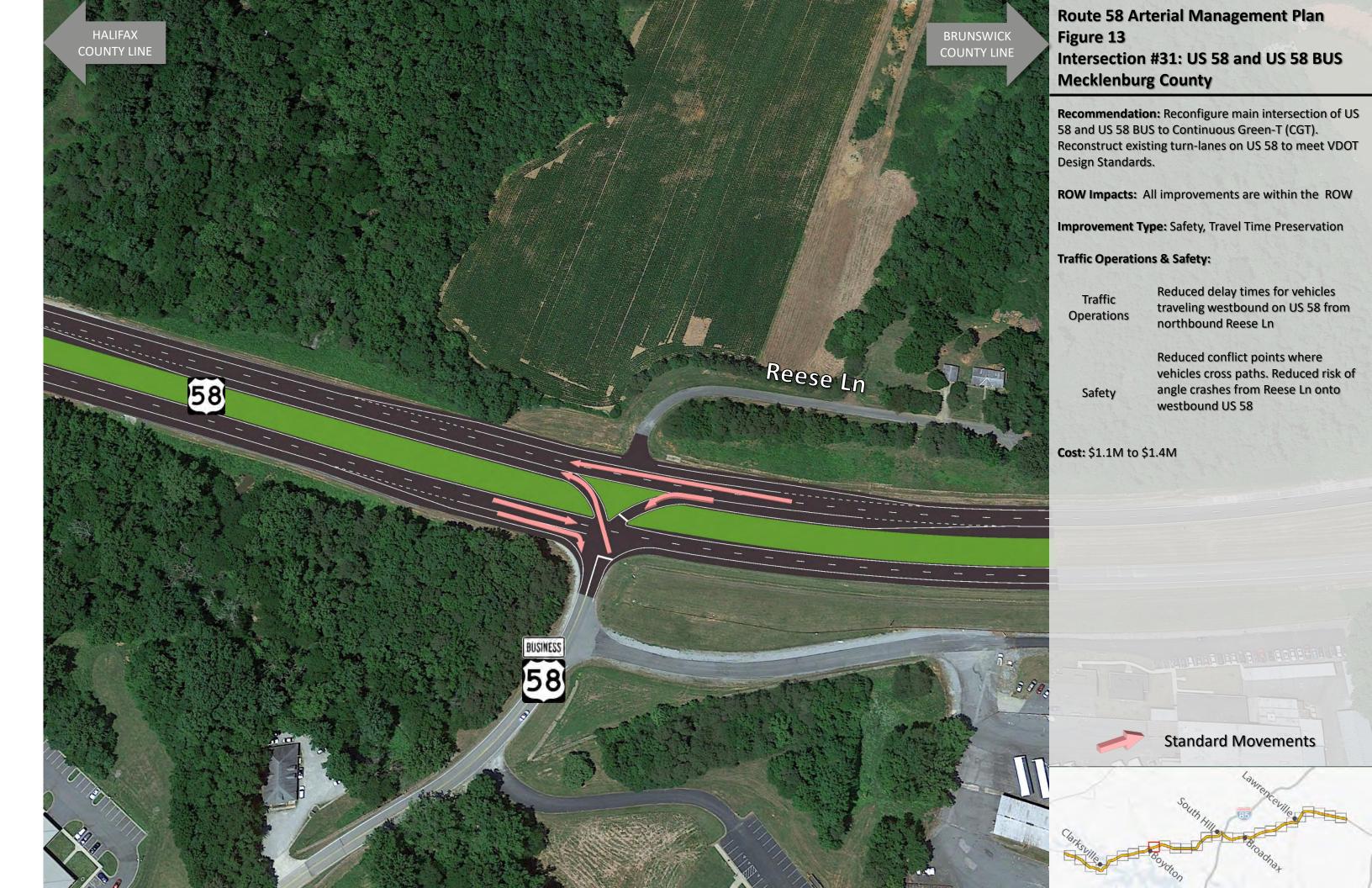
St onto US 58

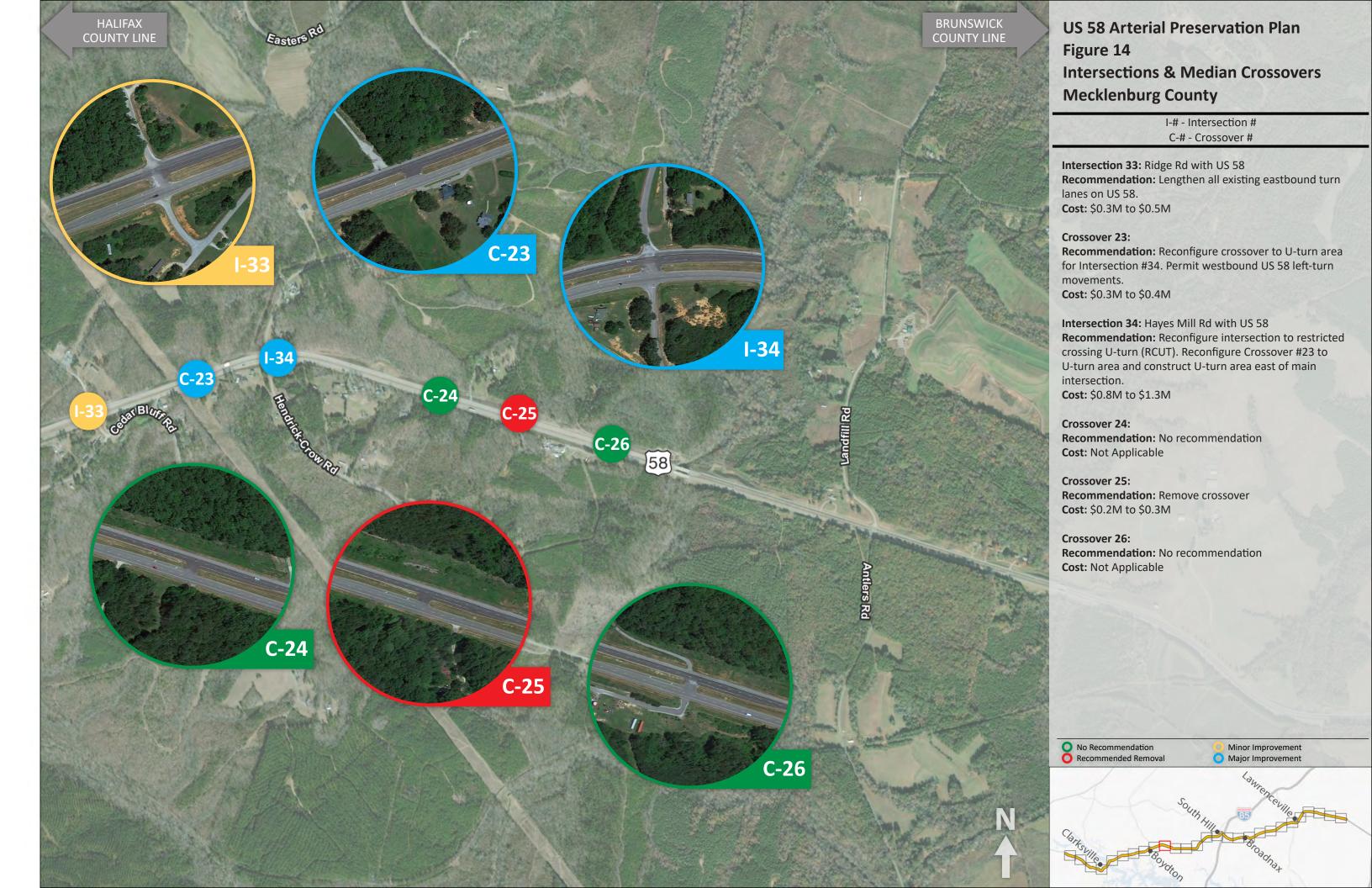
Cost: \$1.2M to \$1.9M



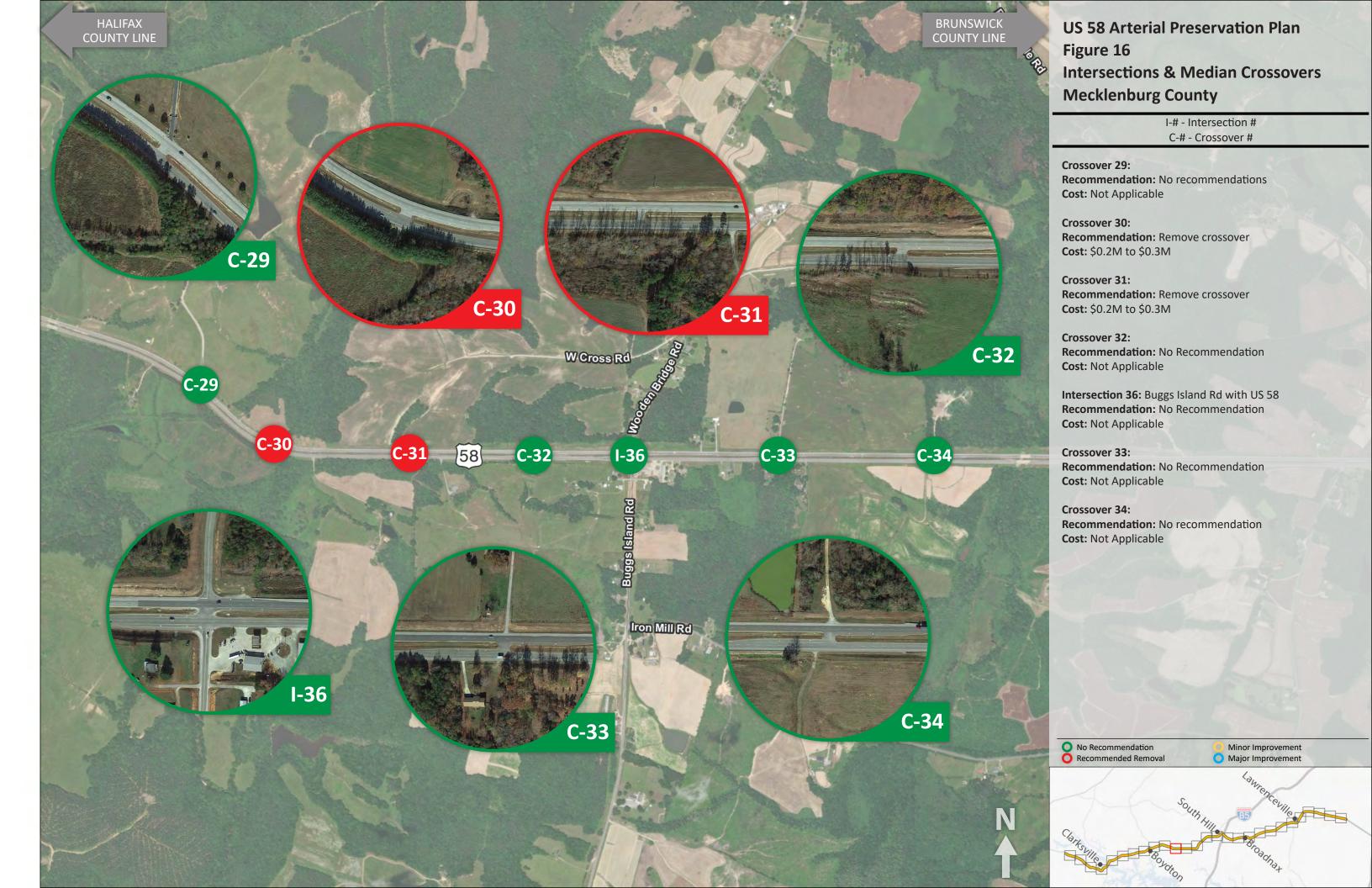
Standard Movements

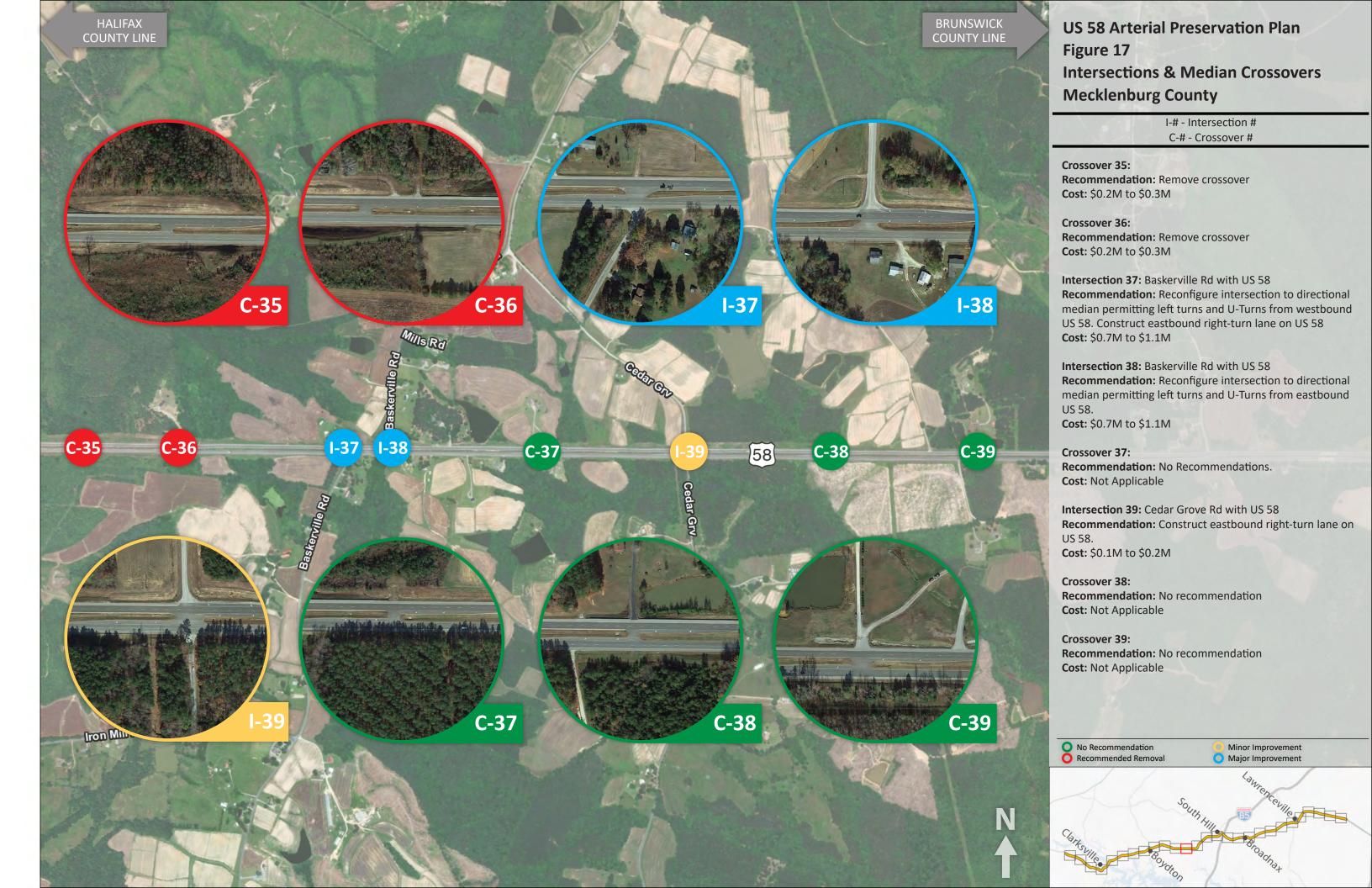
Re-routed Movements















Route 58 Arterial Management Plan Figure 19 Intersection #41: US 58 and US 1 Mecklenburg County

Recommendation: Realign US 1 north-east of existing intersection and reconfigure intersection of US 58 and US 1 to Continuous Green-T (CGT). Construct median between the eastbound and westbound lanes on US 58.

ROW Impacts: CGT is within existing ROW. Although VDOT owns some land east of existing US 58 alignment, the realignment may require additional ROW on the east side of US 58.

Improvement Type: Safety, Travel Time Preservation

Traffic Operations & Safety:

Traffic Operations

Reduced delay times for vehicles traveling westbound on US 58 from northbound US 1

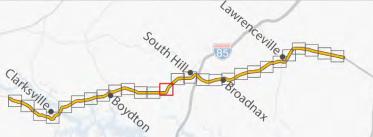
Improved sight distance for all movements at the intersection.

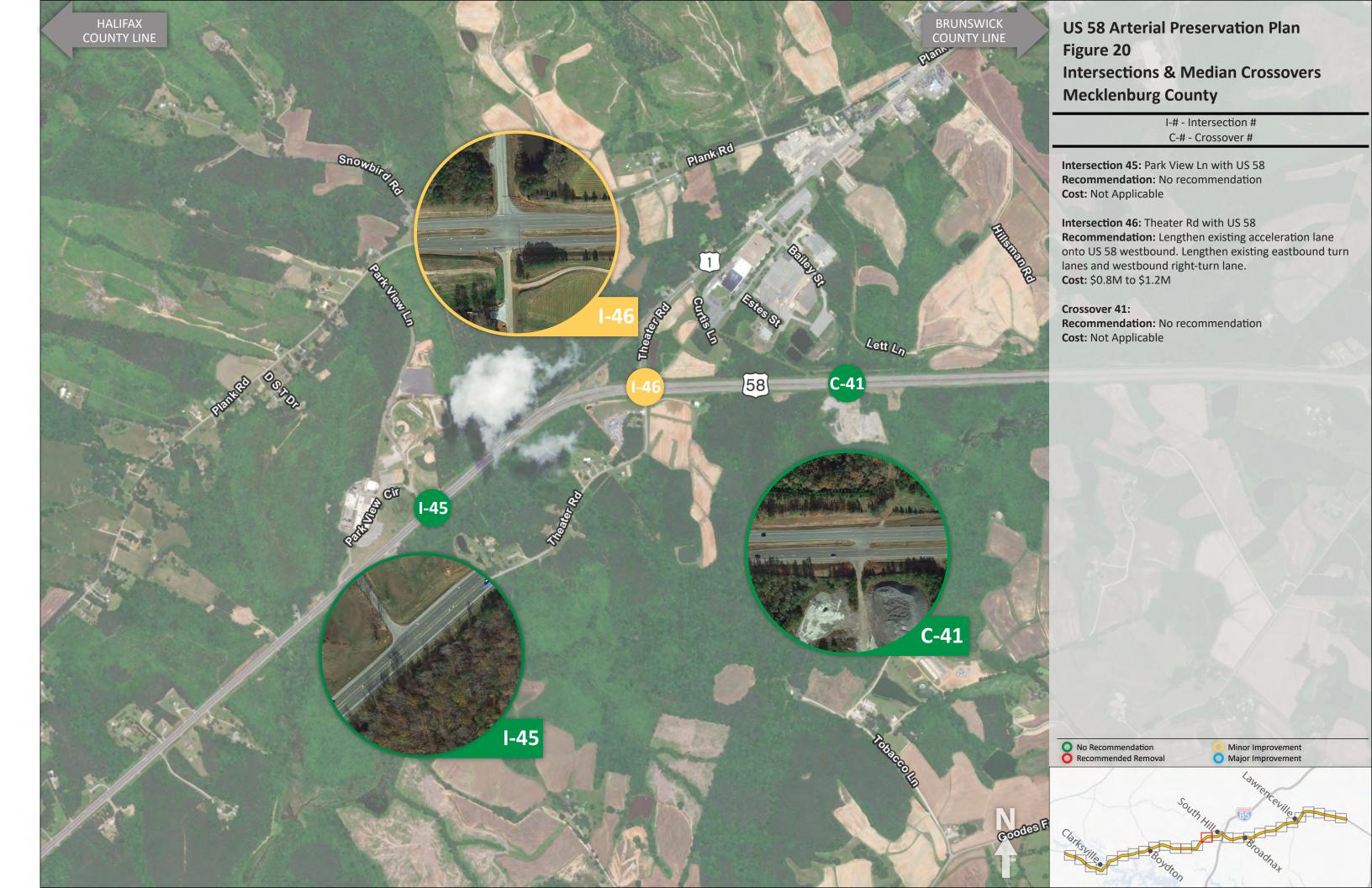
Safety

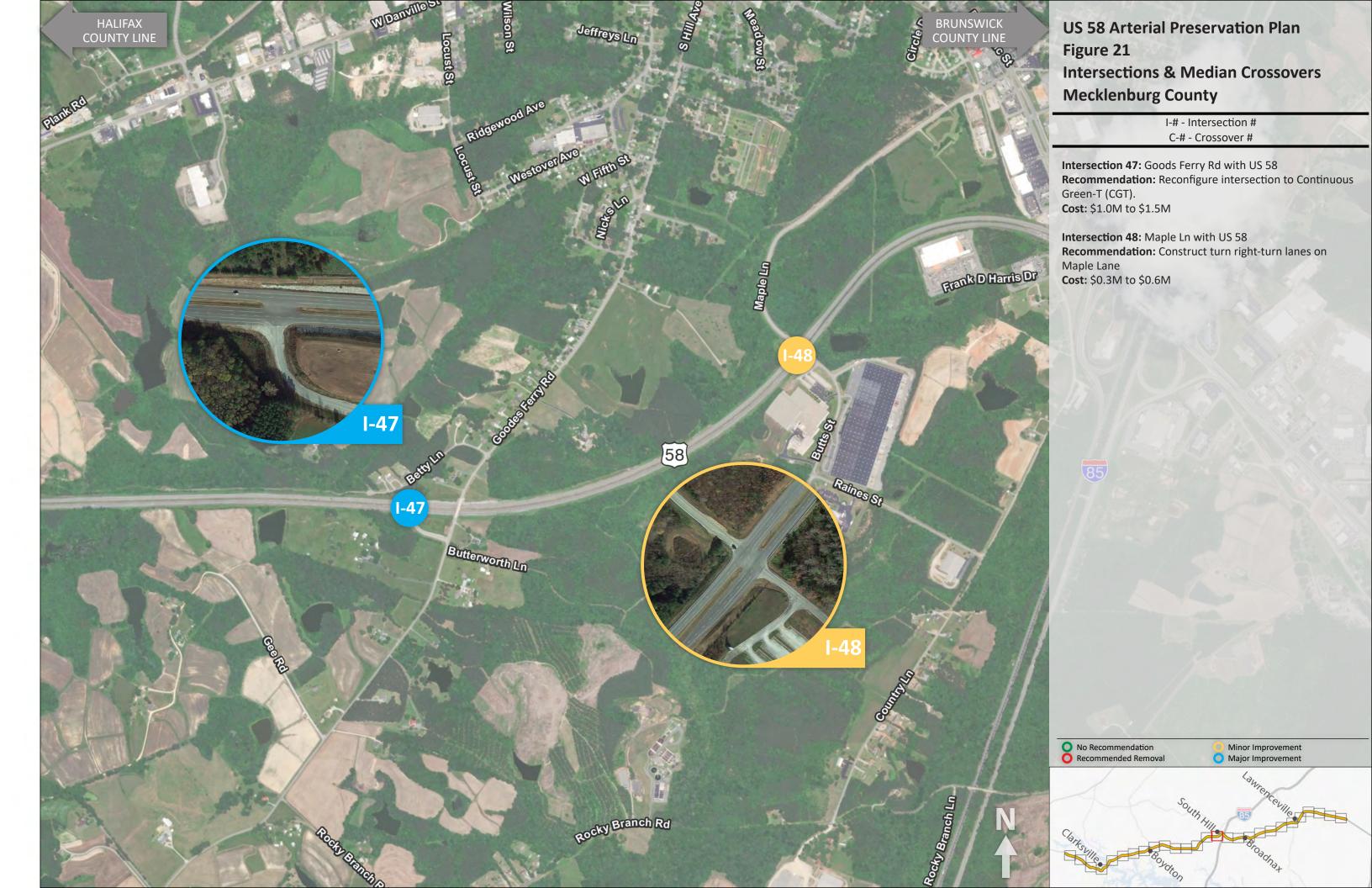
Reduced risk of angle crashes due to CGT and improved sight distance.

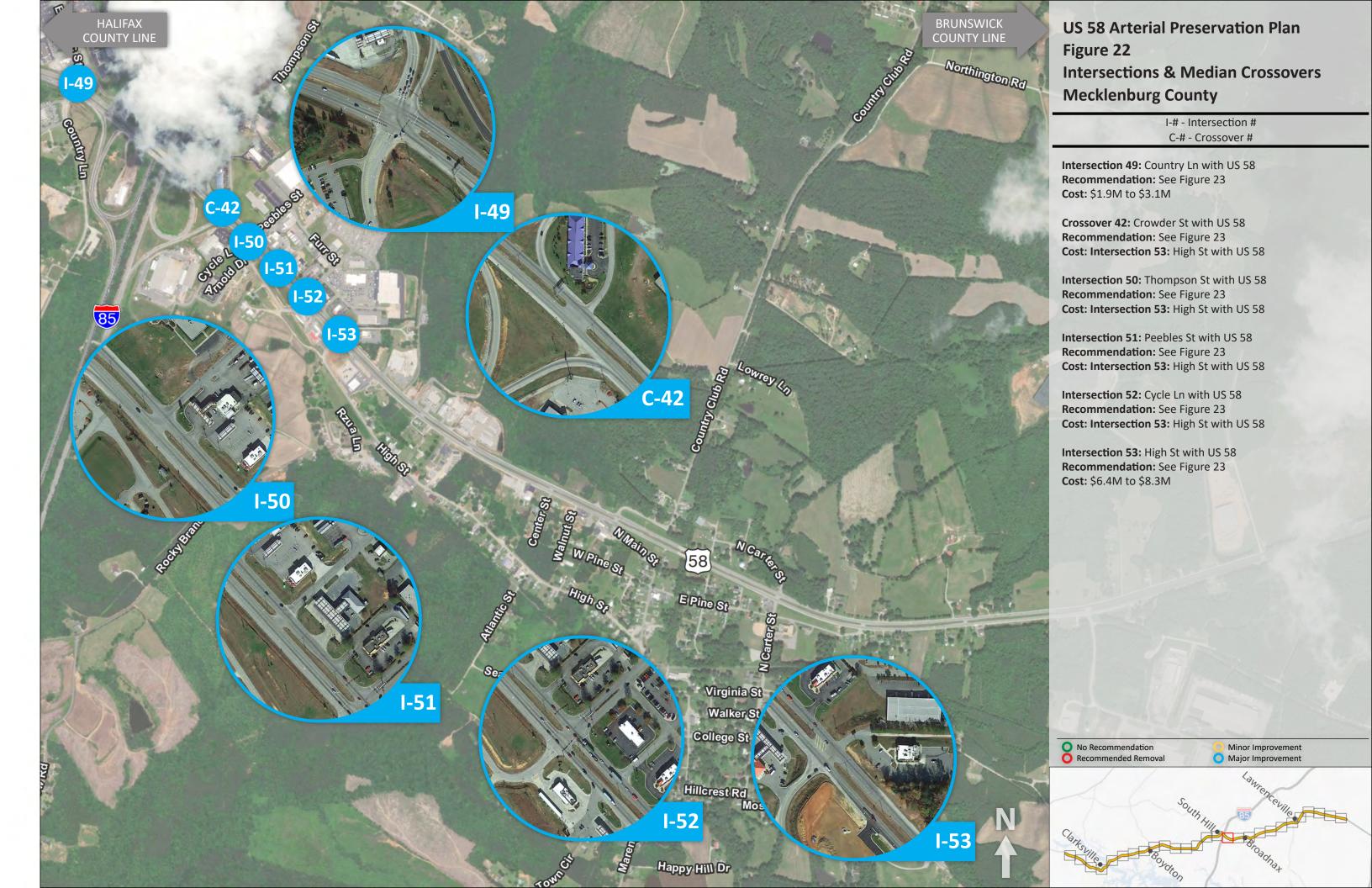
Cost: \$6.9M to \$9.7M

Standard Movements

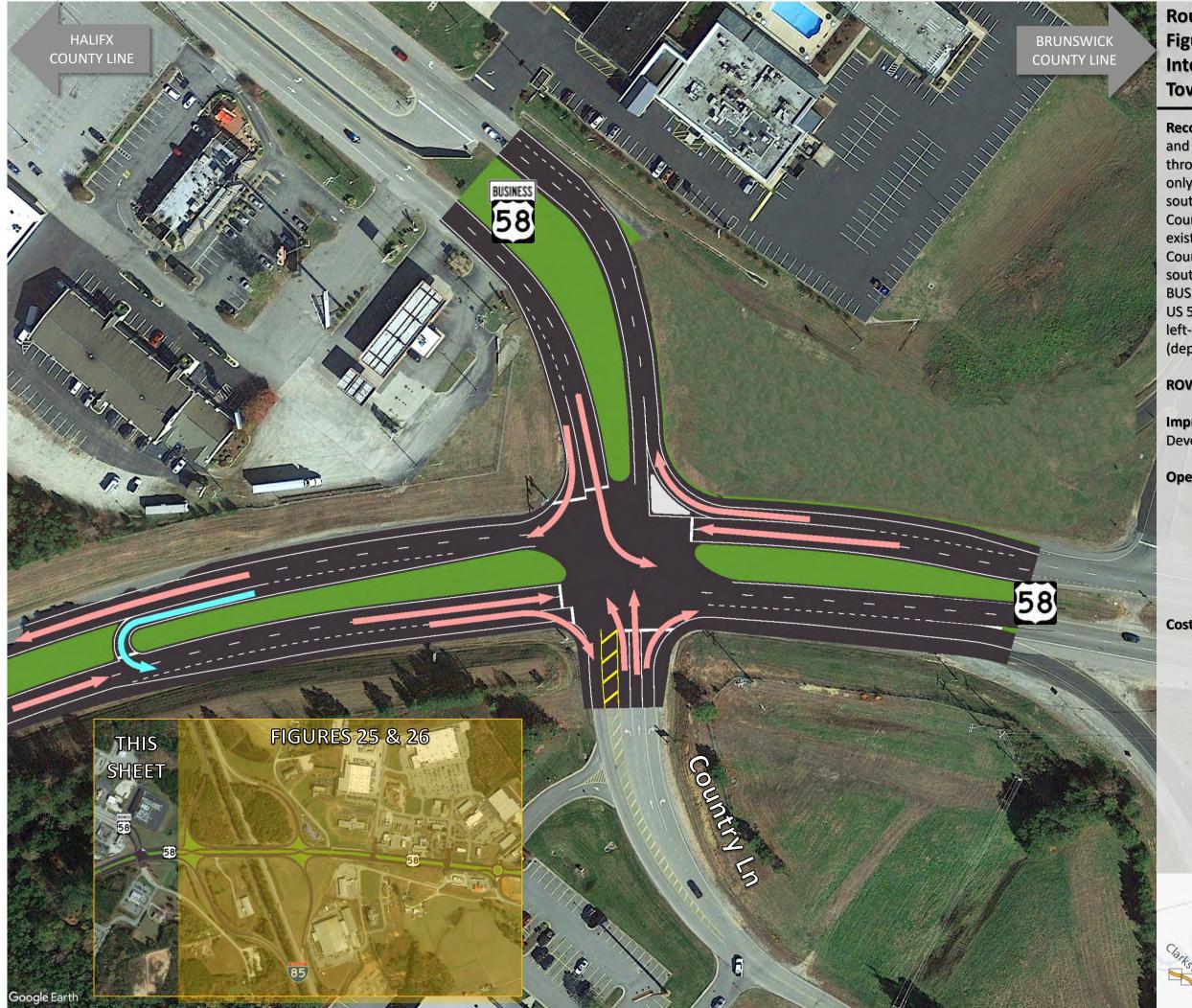












Route 58 Arterial Management Plan Figure 24 Intersection #49: US 58 with Country

Intersection #49: US 58 with Country Ln
Town of South Hill

Recommendation: Reconfigure the existing intersection and traffic signal to a three-phase signal. Permit only through and right-turn movements on US 58. Permit only left and right-turn movements from US 58 BUS southbound onto US 58 and full movements from Country Ln northbound. Construct U-turn area west of existing intersection to permit movements destined to Country Ln from US 58 westbound or US 58 BUS southbound. Remove existing I-85 off ramp onto US 58 BUS and construct continuous flow right-turn lane from US 58 westbound onto US 58 BUS. Eastbound US 58 left-turns to be managed at Maple Lane or interchange (depending on interchange configuration).

ROW Impacts: All improvements are within the ROW

Improvement Type: Congestion Mitigation, Economic Development, Safety, Travel Time Preservation

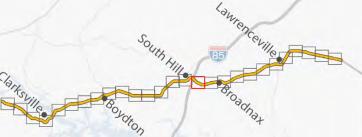
Operations:

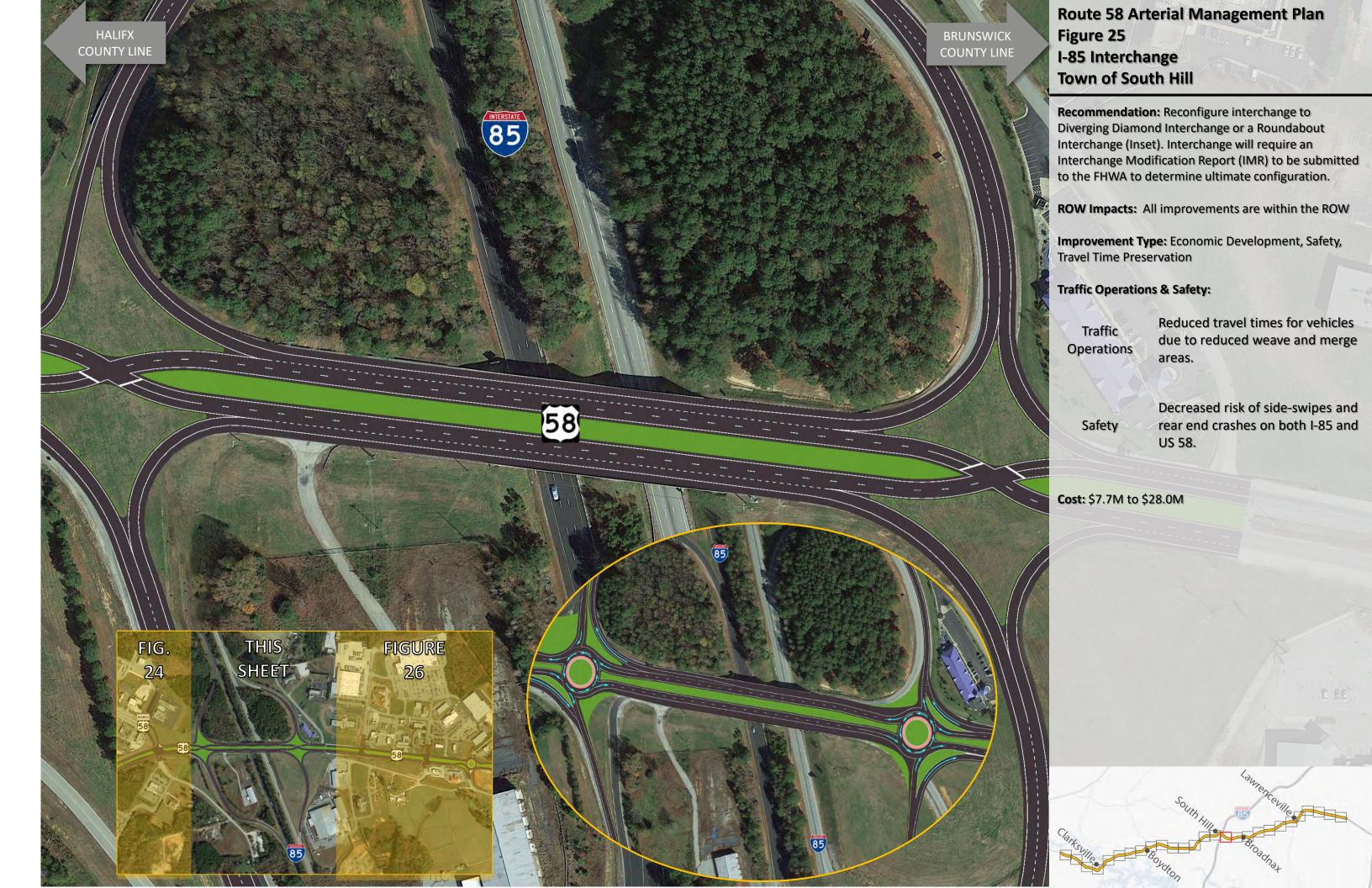
2040 Future Delay (sec - LOS)	No Build	Build
AM	24.6-C	17.4-C
PM	30.4-C	20.4-C

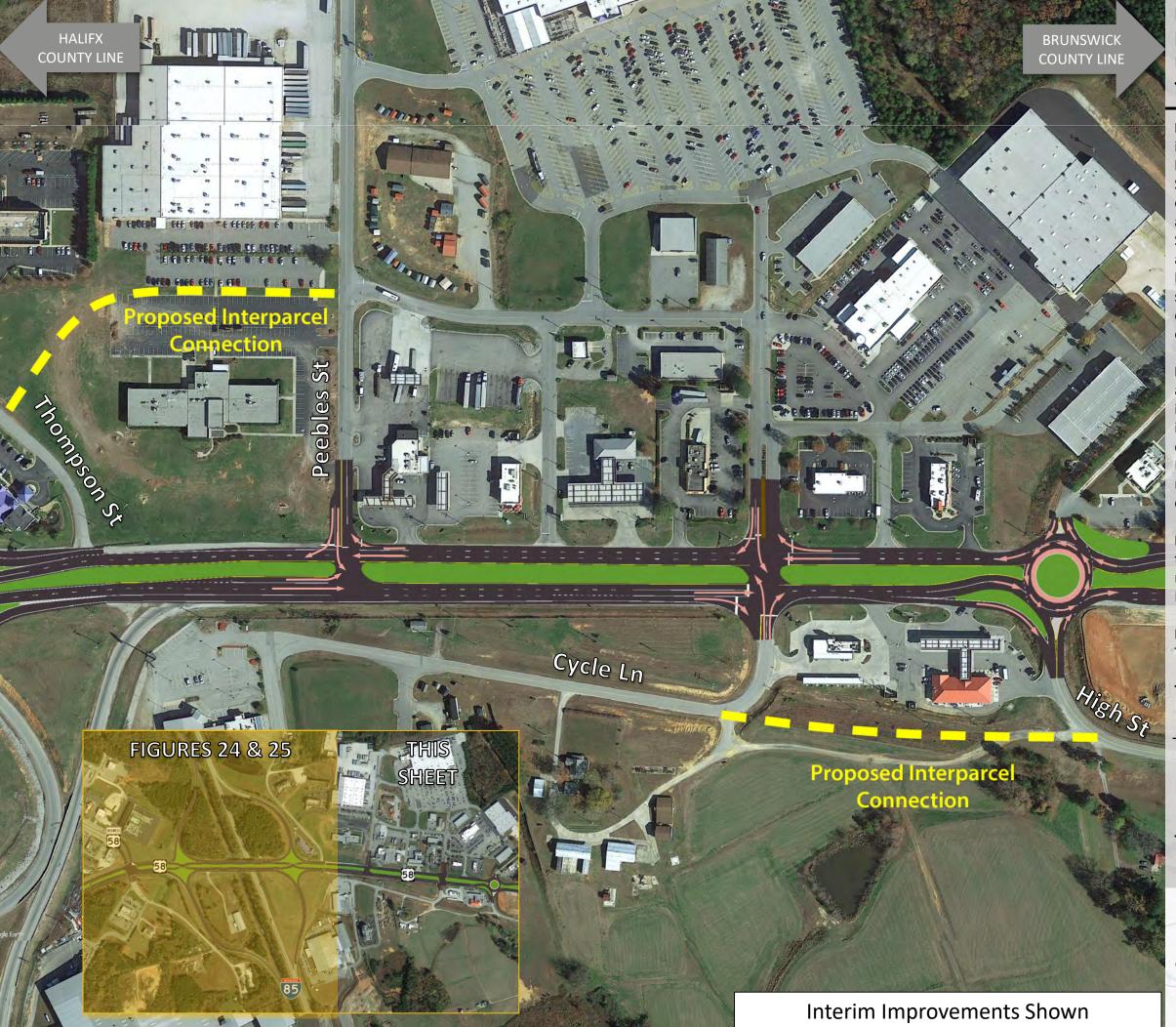
Cost: \$1.9M to \$3.1M



Standard Movements
Re-routed Movements







Route 58 Arterial Management Plan Figure 26 US 58 Eastern Corporate Limits Town of South Hill

Interim Recommendation: Reconfigure Thompson St intersection to right-in/right-out only, improve storage length of eastbound US 58 left-turn lane onto Peebles St, reconfigure Crowder St intersection to right-in/right-out only, reconfigure Cycle Lane to a two-phase signal, and construct a roundabout at the intersection of High St. Construct inter-parcel connections to maintain access between Thompson St and Peebles St, and between Cycle Ln and High St. Town maintained streets should be investigated further to determine pavement condition and capacity improvements to maintain efficient traffic flow.

Long-term Recommendation: As development occurs, additional improvements will be needed at the intersection of Peebles St and US 58. These improvements may require reviews and approvals by district, state, and FHWA officials. The roundabout at High St will need to be reconfigured to remove northbound left and thru movements to maintain capacity of the corridor.

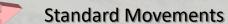
ROW Impacts: All improvements on US 58 are within the ROW. Inter-parcel connections and Town maintained street improvement may require significant ROW acquisition.

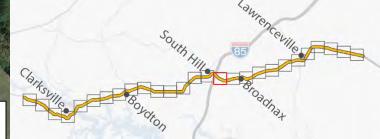
Improvement Type: Economic Development, Safety, Travel Time Preservation

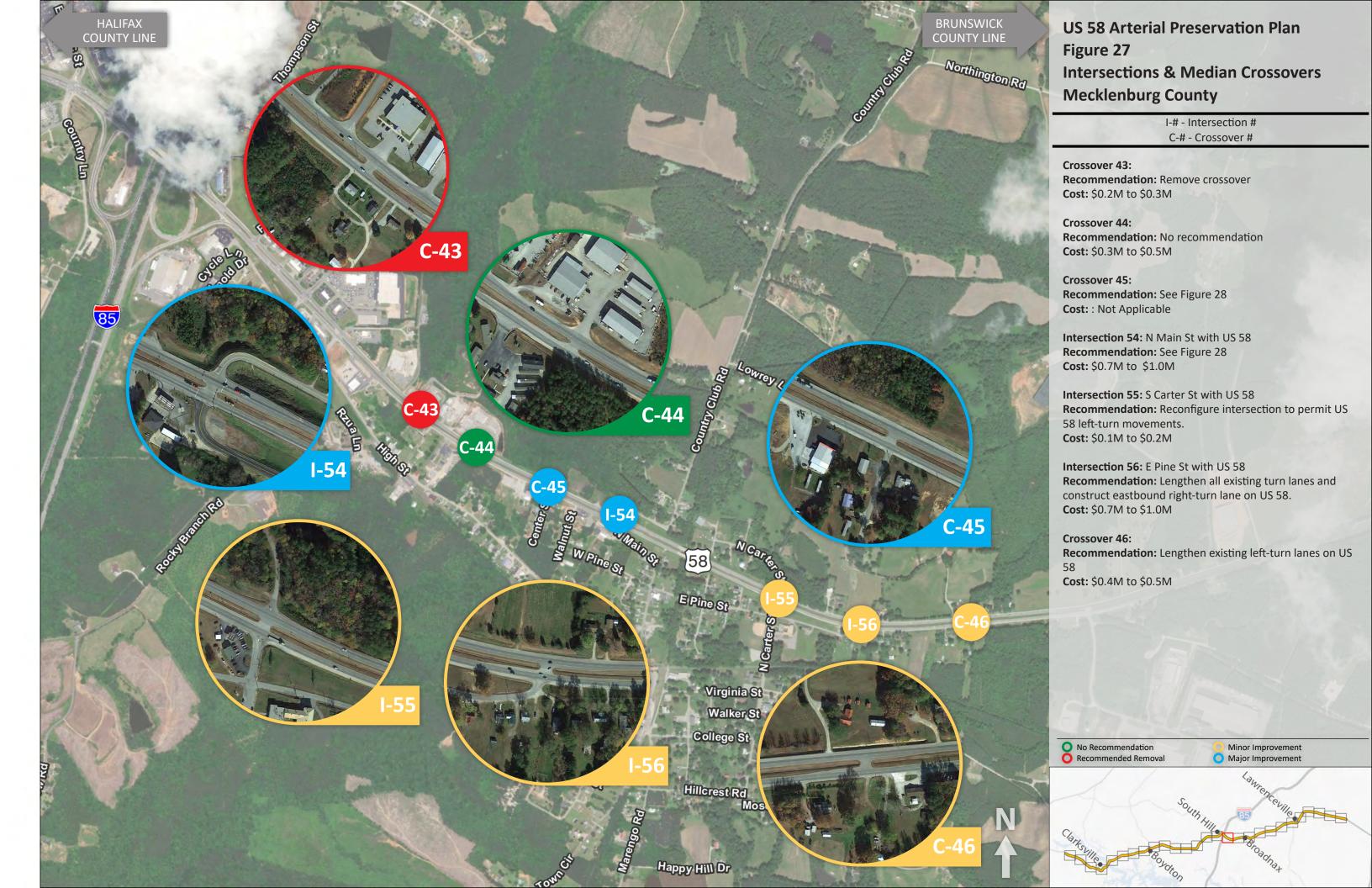
Traffic Operations:

2040 Travel Times (min)	Eastbound US 58		Westbound US 58	
Condition	No Build	Build	No Build	Build
AM	1:02	0:54	1:02	0:58
PM	1:32	1:17	1:38	1:26

US 58 Improvements: \$6.4M to \$8.3M Town Street Improvements: \$1.6M to \$10.0M









Route 58 Arterial Management Plan Figure 28

Intersection #54: N Main St with US 58 **Mecklenburg County**

Recommendation: Reconfigure the existing intersection and traffic signal to a two-phase signal. Permit only through and right-turn movements on US 58. Permit only right-turn movements from Country Club Rd onto US 58 and full movements from northbound N Main St to US 58. Reconfigure Crossover #45 to U-turn area for vehicles destined eastbound US 58 or the Town of LaCrosse. Eastbound US 58 vehicles destined to Country Club Road can use existing Crossover #46.

ROW Impacts: All improvements are within the ROW

Improvement Type: Safety, Travel Time Preservation

Traffic Operations & Safety:

Traffic Operations Reduced delay times for vehicles traveling on US 58 and N Main St.

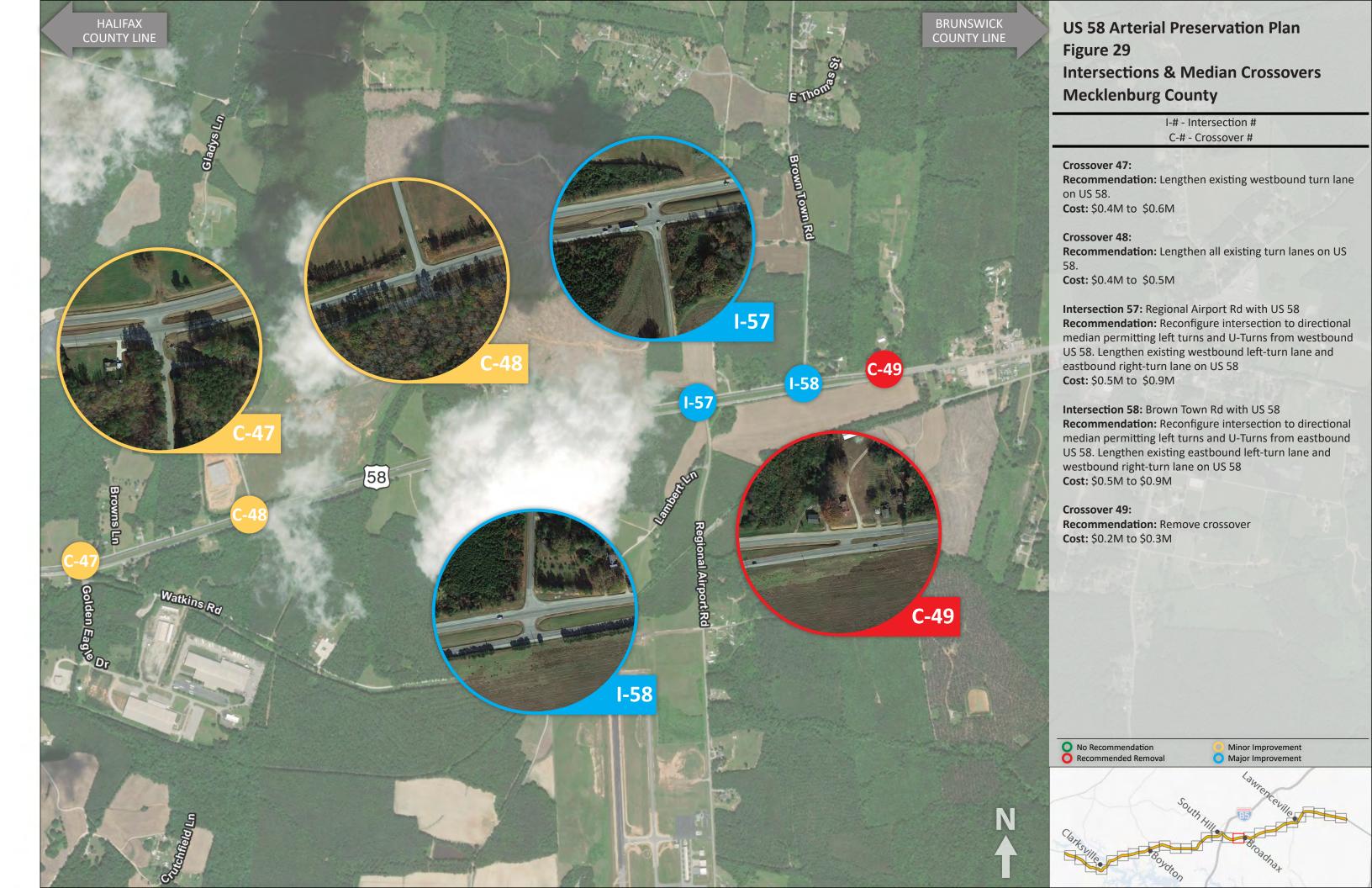
Significant reduction in risk of angle crashes and reduced risk of rear end crashes.

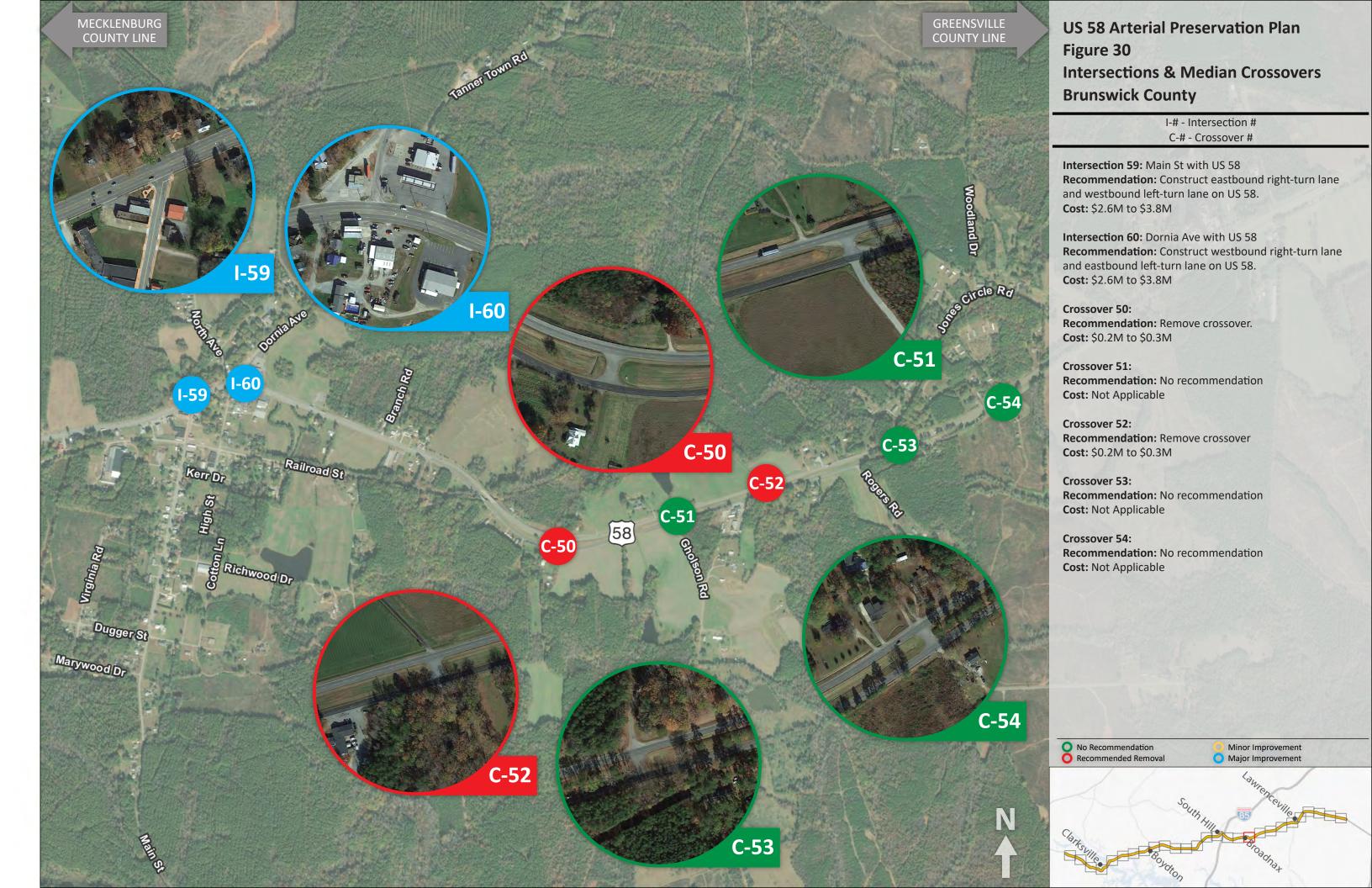
Cost: \$0.7M to \$1.1M

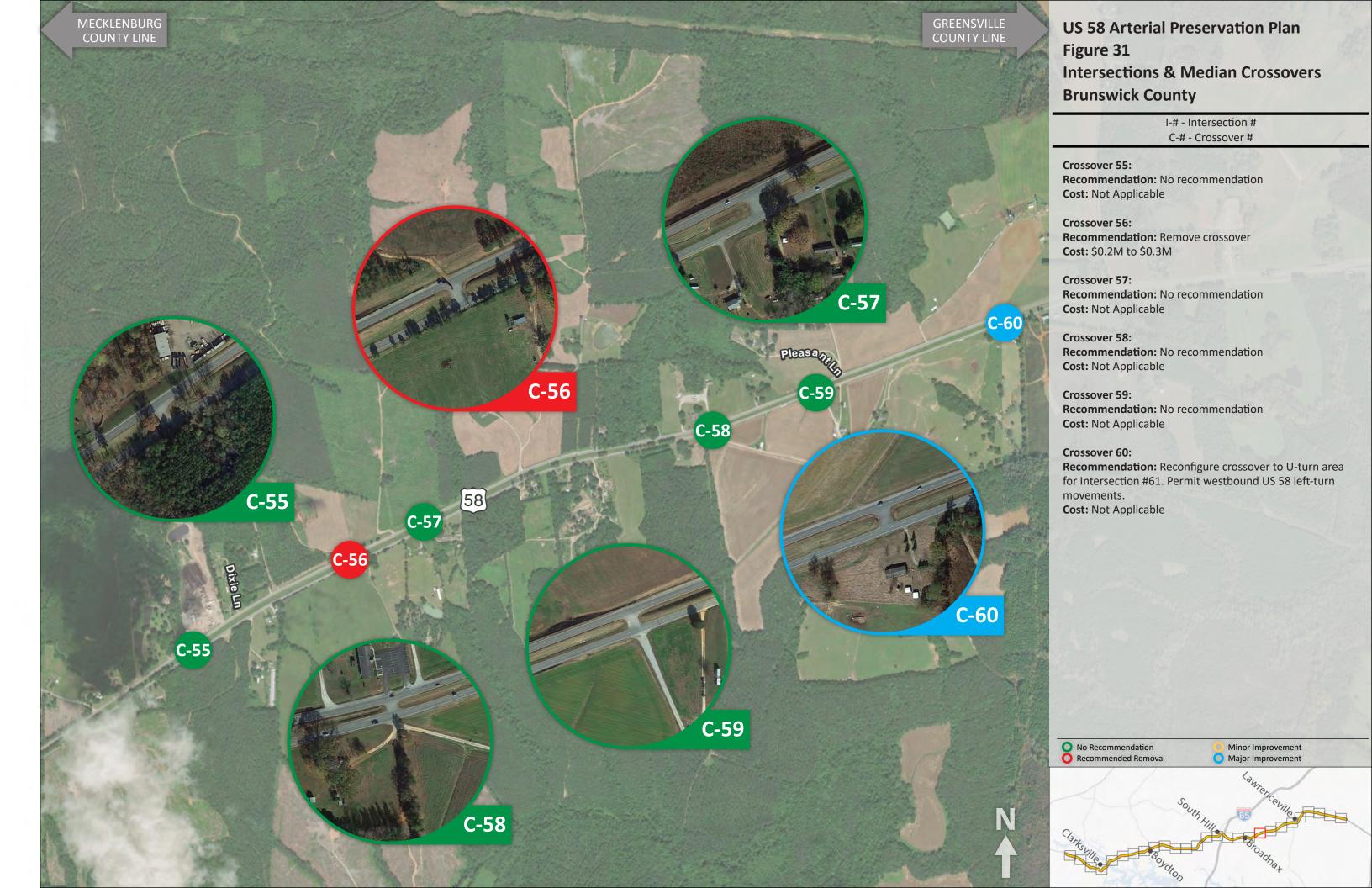


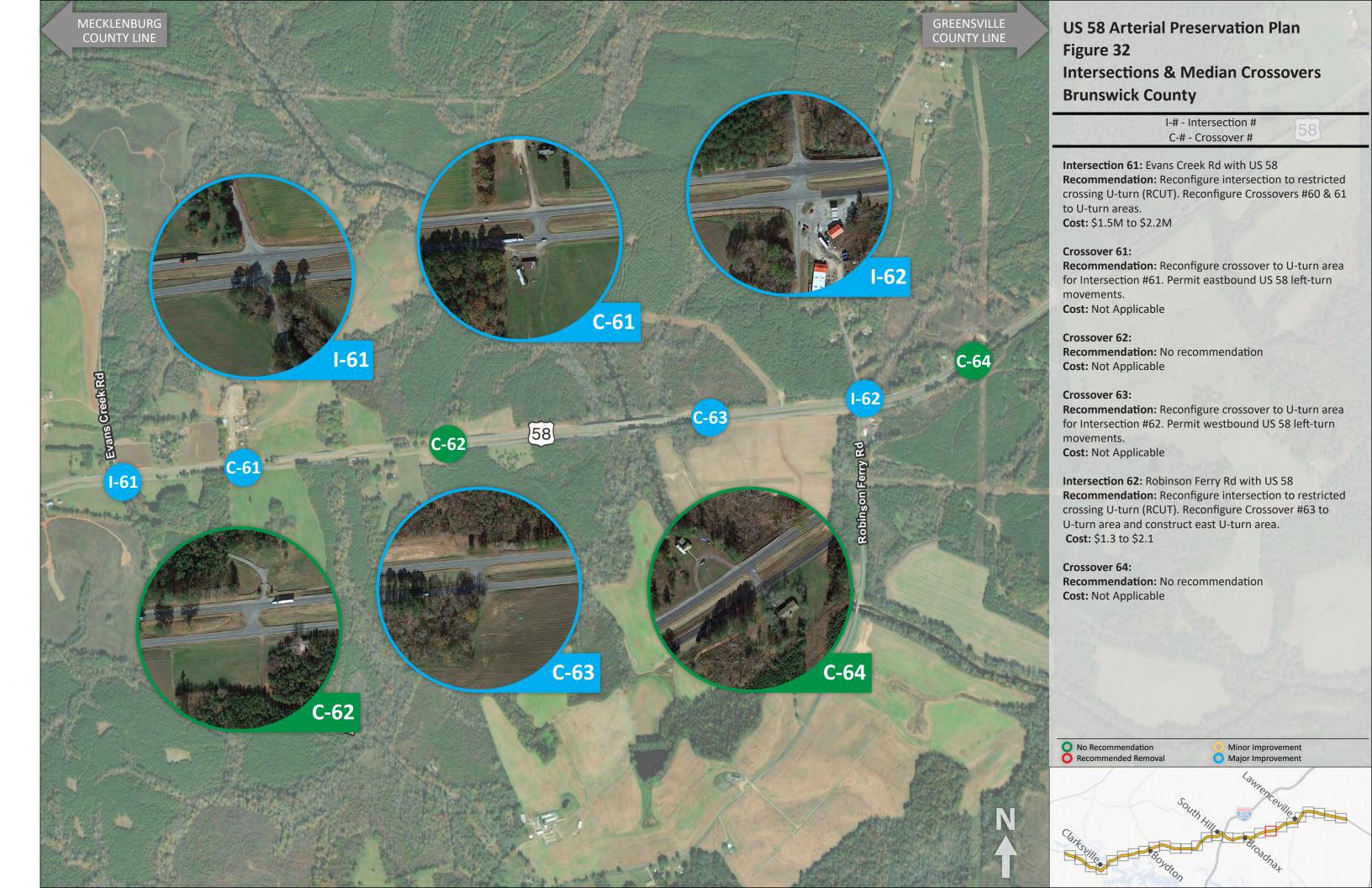
Standard Movements

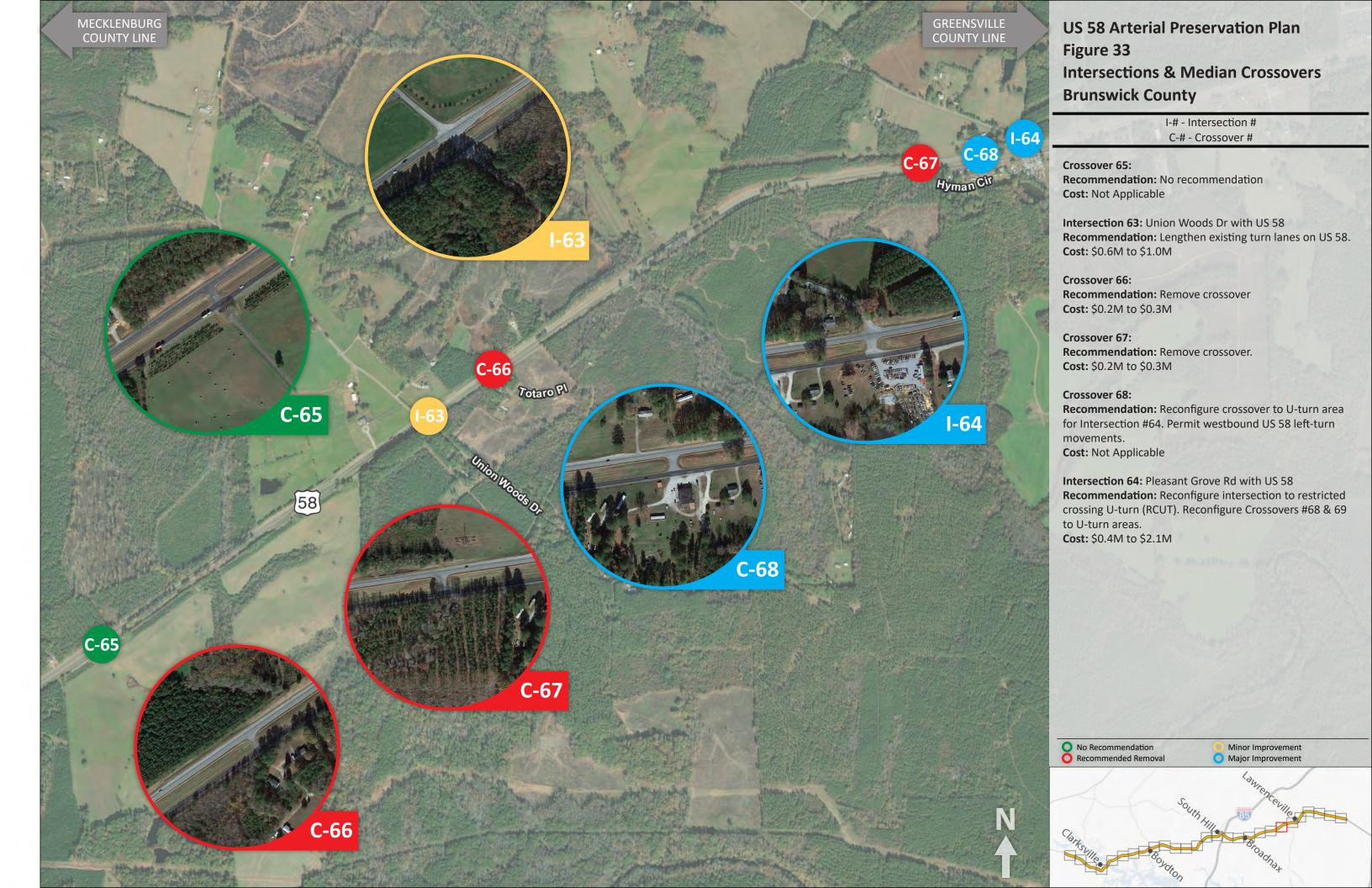
Re-routed Movements

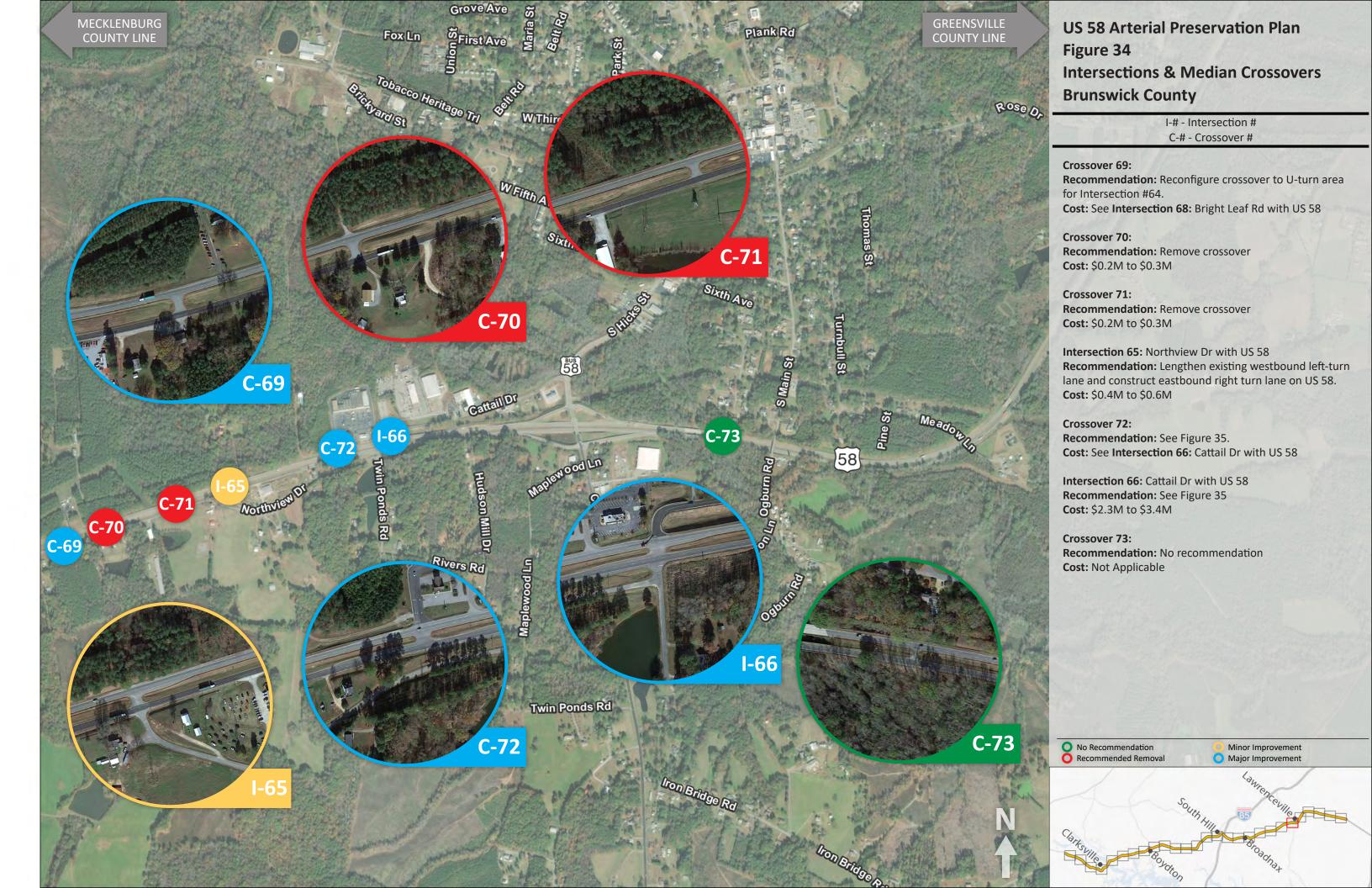


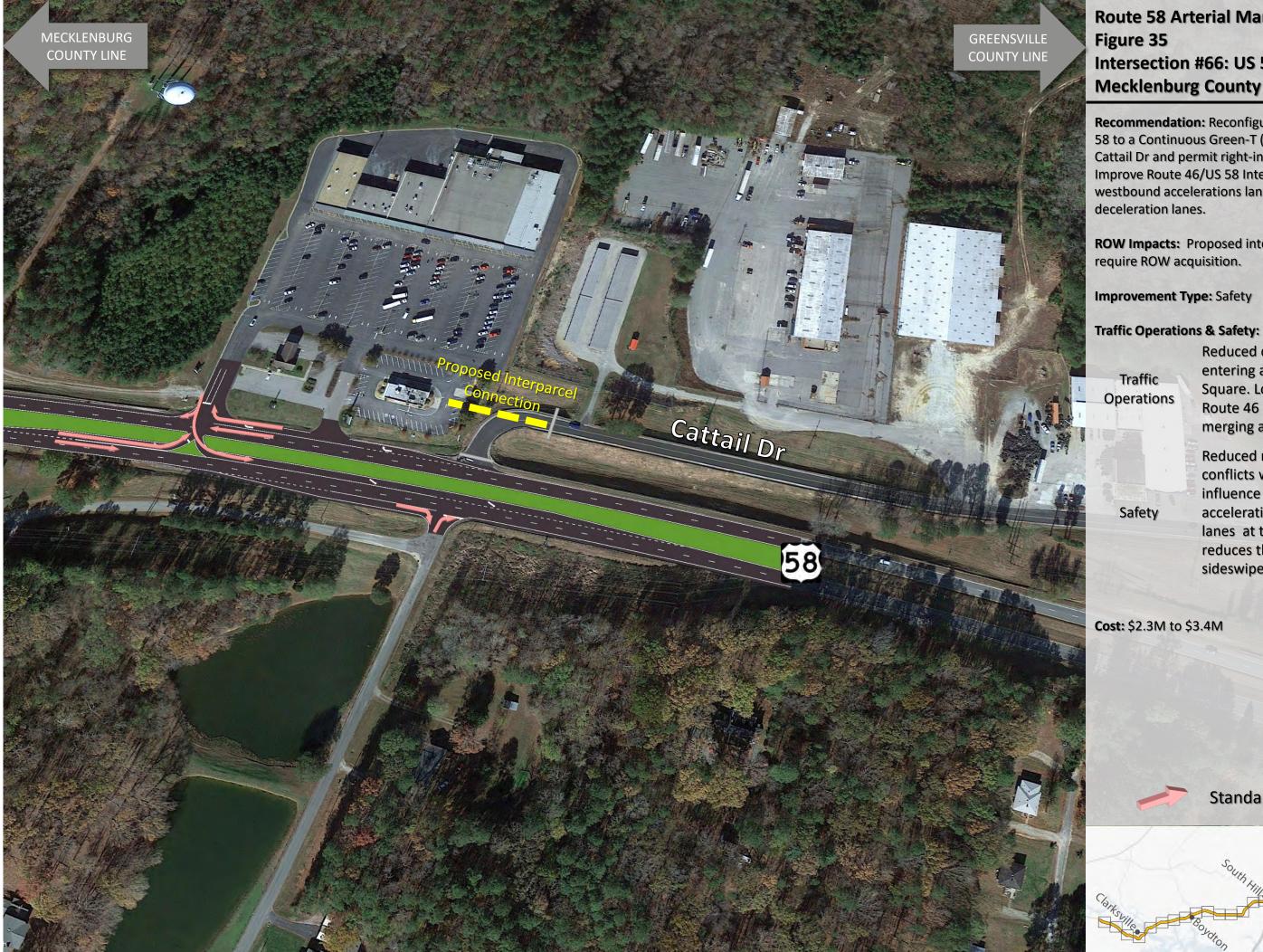












Route 58 Arterial Management Plan Figure 35 Intersection #66: US 58 and Cattail Dr

Recommendation: Reconfigure Crossover #72 with US 58 to a Continuous Green-T (CGT). Close north leg of Cattail Dr and permit right-in/right-out for the south leg. Improve Route 46/US 58 Interchange by extending westbound accelerations lanes and eastbound deceleration lanes.

ROW Impacts: Proposed interparcel connection may require ROW acquisition.

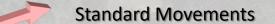
Improvement Type: Safety

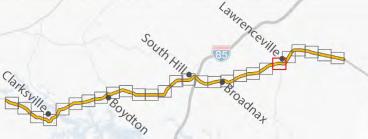
Traffic Operations & Safety:

Traffic Operations Reduced delay for vehicles entering and exiting Brunswick Square. Longer merge area for Route 46 Interchange permits merging at higher safer speeds

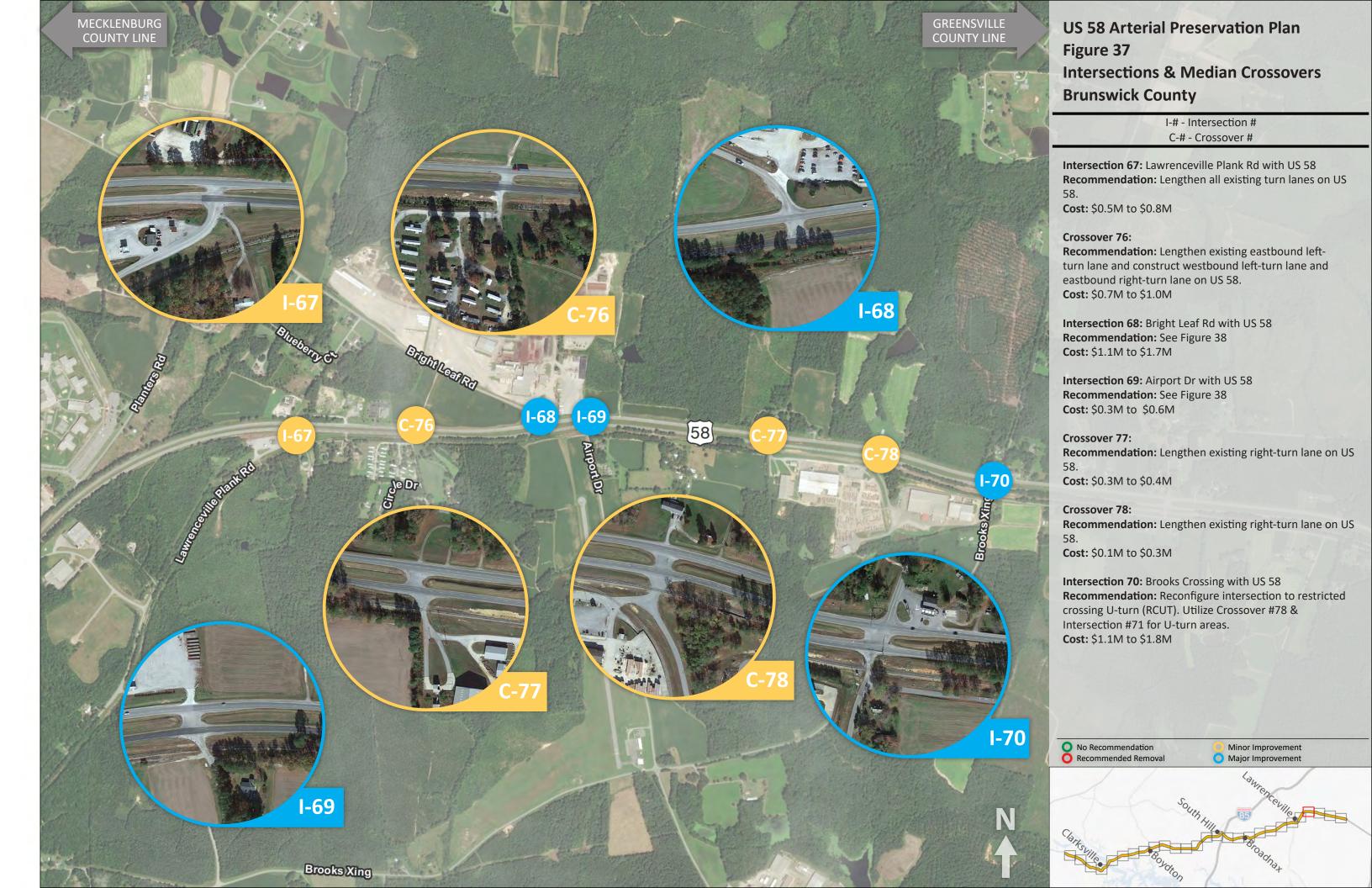
Reduced risk of angle crashes and conflicts with the Route 46 influence area. Longer acceleration and deceleration lanes at the Route 46 interchange reduces the risk of rear end and sideswipe crashes

Cost: \$2.3M to \$3.4M











Route 58 Arterial Management Plan Figure 38

Intersection #68: US 58 and Bright Leaf Rd Intersection #69: US 58 and Airport Dr Mecklenburg County

Recommendation: Reconfigure intersection of Bright Leaf Rd with US 58 to a Continuous Green-T (CGT). Construct U-turn area west of main intersection to permit movements from Bright Leaf Rd or US 58 westbound to Airport Dr. Reconfigure the intersection of Airport Dr with US 58 to right-in/right-out. Construct U-turn area east of Airport Dr to permit movements from Airport Drive to Bright Leaf Rd or US 58 westbound. A restricted crossing U-turn (RCUT) alternative is also applicable for this location.

ROW Impacts: Most improvements are within the ROW. The extended eastbound right-turn lane may have little to no ROW impacts.

Improvement Type: Safety

Traffic Operations & Safety:

Traffic Operations

Reduced delay for vehicles exiting Bright Leaf Rd and Airport Drive.

Safety

Reduced risk of angle crashes at both the intersections of Bright Leaf Rd and Airport Dr with US 58. Increased sight-distance for turning vehicles. Eliminates trucks stopped in the median blocking traffic.

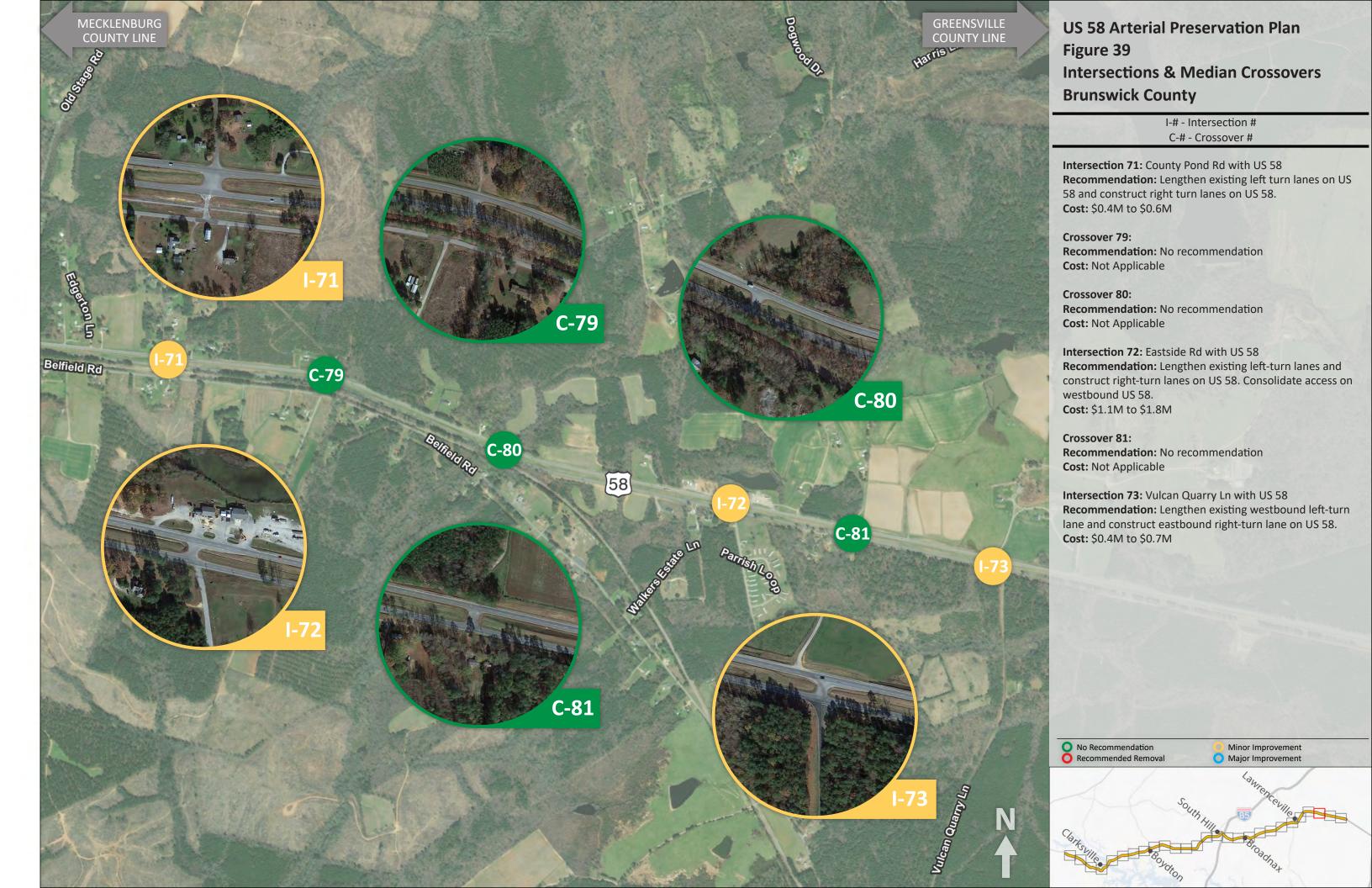
Cost: \$1.4M to \$2.3M

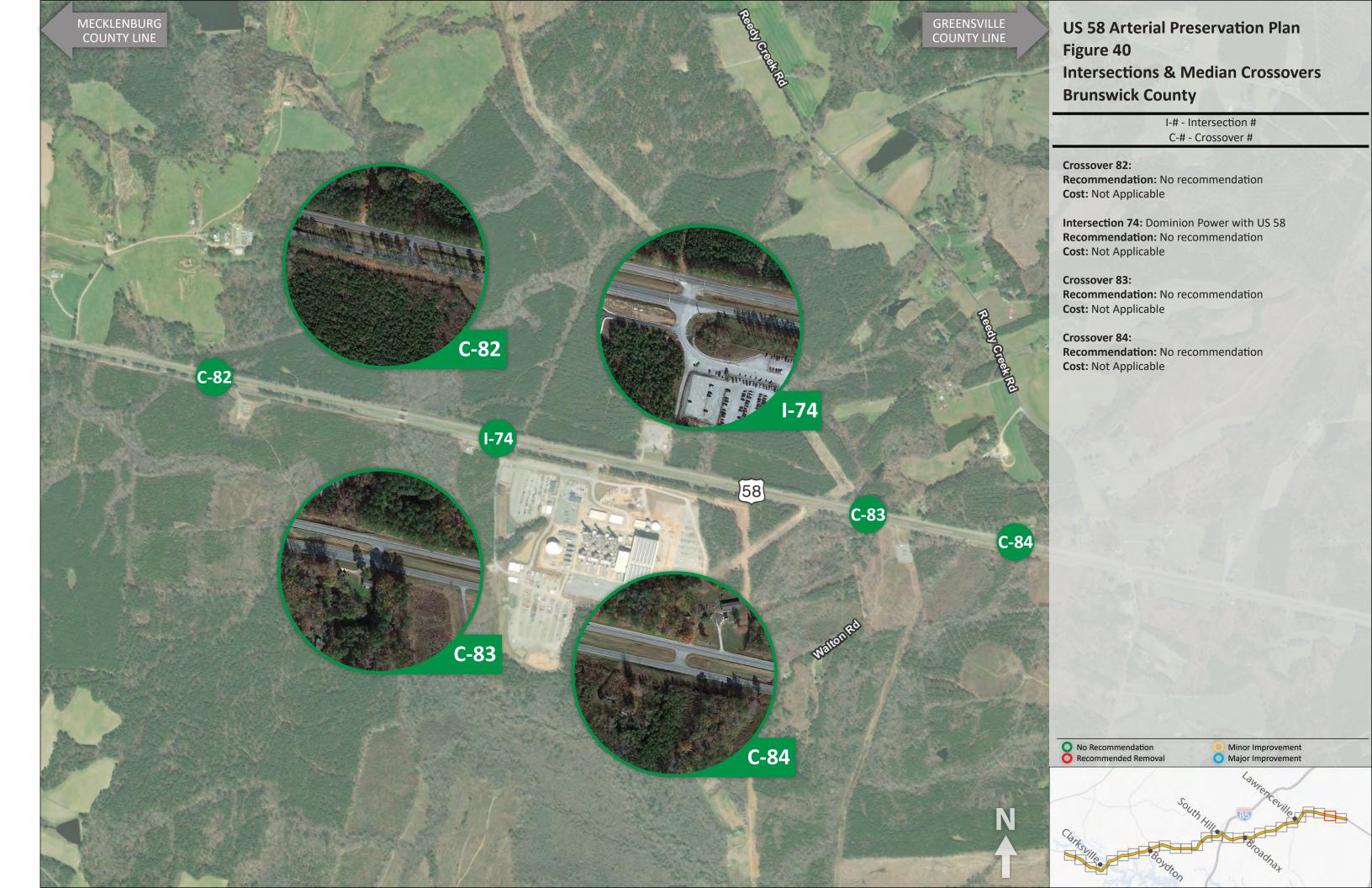


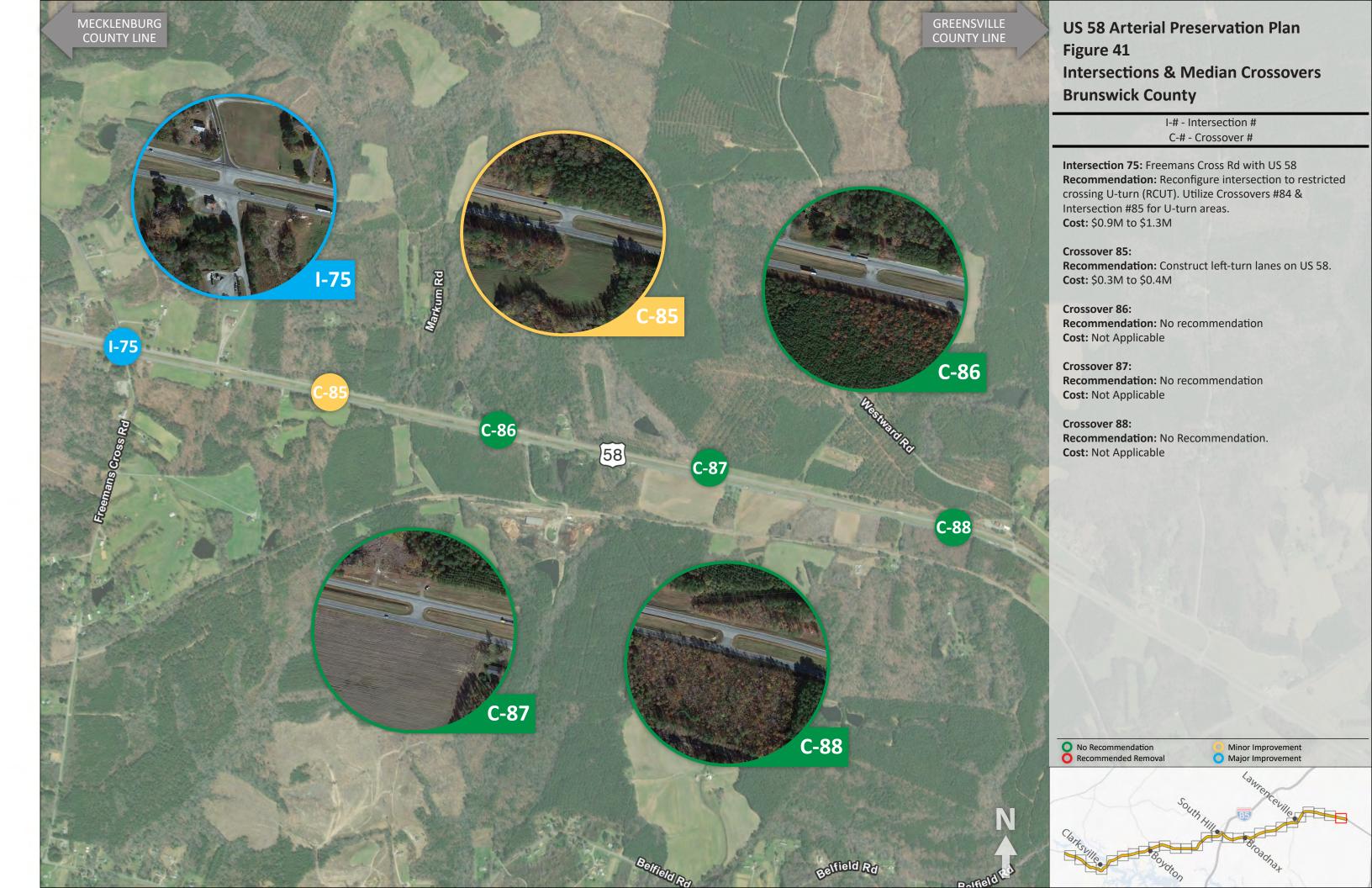
Standard Movements

Re-routed Movements









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APPENDIX B: FIELD REVIEW

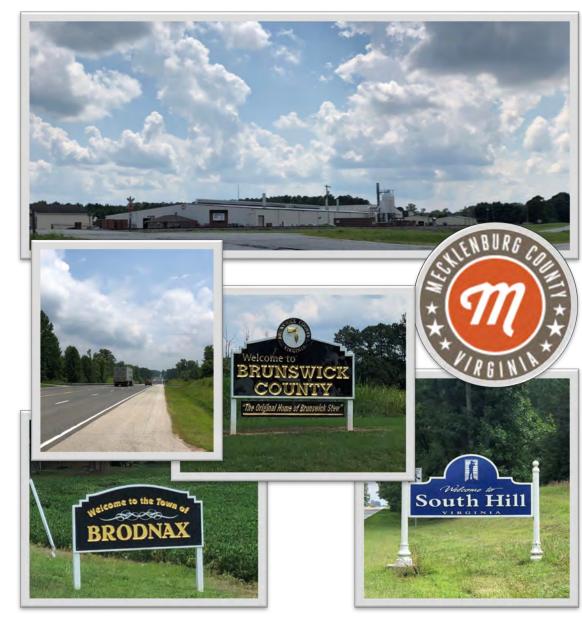
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US 58 Field Review

Conducted: June 12, 2018

Objectives:

- Review roadway and intersection configurations
- Identify deficiencies and areas of concern
 - Sight distance or steep grades
- Identify unique roadway features
- Observe traffic operations





- Rolling terrain
- Intersection and crossovers with sub-standard turn lanes
- Mix of shoulder type
 - Portions without a paved shoulder
 - Widens to 4ft (EB) and 8ft (WB) gravel shoulders
- Grade differentials between EB and WB at intersections and crossovers

US 58 Business to US 15 Interchange

- Partial controlled access
- Close proximity of Rte. 723 intersection to interchange
- 8ft paved shoulders









- Level terrain
- 8ft paved shoulders
- Limited access section around Clarksville
- Some closely spaced intersections
- Rte. 92 Intersection controlled with flashers
 - Limited sight distance for EB approach
 - WB right-turn lane is sub-standard

• Rte. 92 (Washington St.) to Rte. 4 (Buggs Island Rd.)

- Numerous crossovers and increasing access on this segment
- Rte. 4 intersection has steep inclines on both US 58 approaches. May limit sight distance on side streets









- More rolling terrain may be problematic for heavy vehicles
- Minor intersections with sight distance concerns
- Redundant crossovers
- Cross slope at US 1 intersection not ideal for trucks

• Western US 1 Intersection to Rte. 780 (Theater Rd.)

- TWLTL between US 1 intersections
- Increase in direct access to US 58
- Accel lane onto WB US 58 at eastern US 1 intersection appears short.
- Some sight distance concerns at Rte. 780 intersection









- 8' 10' paved shoulders
- Minimal access and crossovers
- Rte. 643 intersection in close proximity to I-85 interchange

• I-85 / US 58 Interchange

- SB I-85 to WB US 58 has improper through movement pavement marking leading to a small gravel section in the median
- T-intersection for SB I-85 to WB US 58 may be problematic for trucks entering the roadway
- No turn lane or taper for WB US 58 to NB I-85









- Motorists cutting across EB US 58 from NB I-85 off ramp to entrance of Shell gas station. (Less than 600')
- Numerous access points and median crossovers
- Shaw St. / Cycle Ln. has a significant grade disparity between EB and WB US 58
- Rte. 621 intersection:
 - Side street approaches have steep grades and sharp curves
 - Lots of access points around intersection

• Rte. 621 (N. Main St.) to Town of Brodnax

- Inconsistent shoulder width and type
- Crossovers lacking turn lanes
- Rolling terrain









- TWLTL throughout
- Lots of direct access
- 4' grass / gravel shoulders

• Town of Brodnax to Rte. 644 (Robinson Ferry Rd.)

- Variable shoulder width and type
- Crossovers without turn lanes
- Sight distance concerns to some crossovers due to rolling terrain
- Sub-standard turn lanes and a cross slope through the intersection with Rte. 644

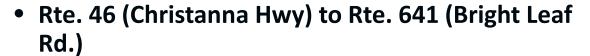








- Crossovers without turn lanes
- Sight distance concerns to some crossovers due to rolling terrain
- 4' paved or gravel shoulders



- Limited Access until near Rte. 641
- Sub-standard turn lanes for crossovers on the eastern portion
- Rte. 641 intersection:
 - Skewed intersection
 - Wide median may make it difficult for trucks









- Lots of commercial activity
- Redundant crossovers and many lack turn lanes
- Rte. 712 intersection:
 - Controlled by overhead flashers
 - Grade differential between EB and WB US 58 may be difficult to cross
 - Decision making may be difficult due to width of median

• Eastside Rd. to Brunswick Co. Line

- Rolling Terrain
- Varying should types and widths





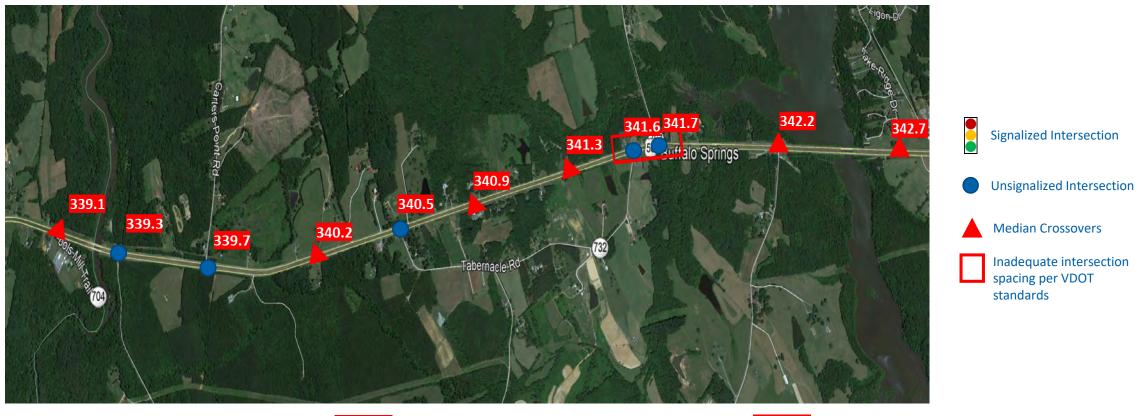


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APPENDIX C: INFRASTRUCTURE INVENTORY

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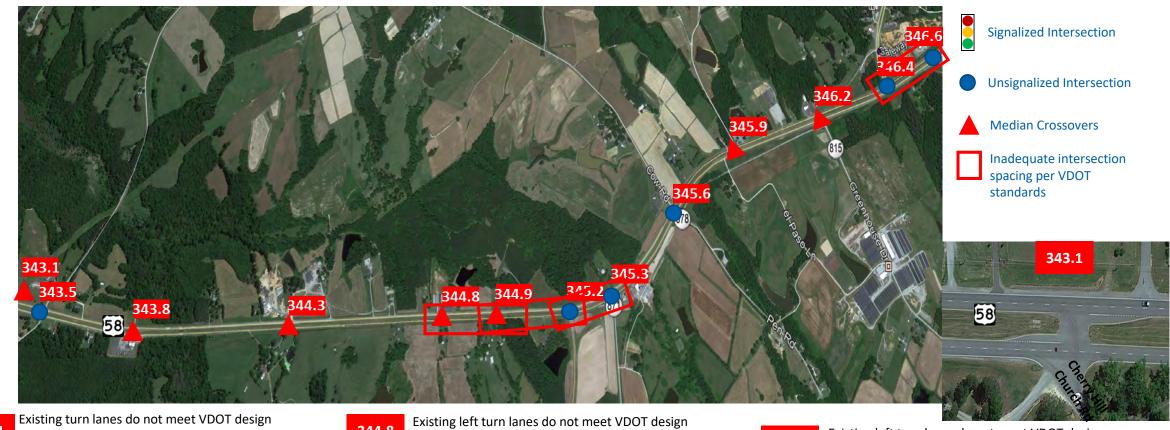
Infrastructure Inventory



- Existing left turn lanes do not meet VDOT design requirements (storage length).
- Existing left turn lanes do not meet VDOT design requirements. No US 58 right turn lanes. Grade differential.
- Existing left turn lanes do not meet VDOT design requirements. Grade differential.
- Existing left turn lanes do not meet VDOT design requirements. No US 58 right turn lanes.

- Existing left turn lanes do not meet VDOT design requirements. No US 58 right turn lanes.
- Existing left turn lanes do not meet VDOT design requirements.
- Existing left turn lanes do not meet VDOT design requirements. Truck U-Turn tire marks.
- Existing left turn lanes do not meet VDOT design requirements. No eastbound US 58 right turn lane

- Existing left turn lanes and southbound right turn lane do not meet VDOT design requirements. Ambulatory injury
- Existing left turn lanes do not meet VDOT design requirements. Existing Eastbound US 58 Right turn lane does not meet VDOT design requirements. Ambulatory Injury
- Existing left turn lanes do not meet VDOT design requirements.



Existing turn lanes do not meet VDOT design requirements. Skewed intersection, Multiple street and driveway accesses. (High crash point Fatal and Ambulatory).

All existing turn lanes do not meet VDOT design requirements.

Existing left turn lanes do not meet VDOT design requirements. Visible injury

Existing left turn lanes do not meet VDOT design requirements. No US 58 Right turn lanes.

Existing left turn lanes do not meet VDOT design requirements.

Existing left turn lanes do not meet VDOT design requirements

Existing left turn lanes do not meet VDOT design requirements. Grade difference. Visible injury.

Existing left turn lanes do not meet VDOT design requirements. Ambulatory injury. Access management.

Existing left turn lanes do not meet VDOT design requirements. No US 58 Right turn lanes. Visible injury.

Existing left turn lanes do not meet VDOT design requirements.

Existing left turn lanes do not meet VDOT design requirements. No US 58 Right turn lanes.

Existing left turn lanes do not meet VDOT design requirements. (Ambulatory Injury)

Existing left turn lanes do not meet VDOT design requirements.



All Existing turn lanes do meet VDOT design requirements. Some vertical curve sight issues.

348.4

349.0

No left turn lanes exist at this median crossover

No left turn lanes exist at this median crossover.

All existing turn lanes do meet VDOT design requirements.

351.3 Existing turn lanes do meet VDOT design requirements.

All existing turn lanes do meet VDOT design requirements. Ambulatory injury.

Existing turn lanes do meet VDOT design requirements.

No US 58 right turn lanes.

352.6 Existing turn lanes do meet VDOT design requirements.

58

All Existing turn lanes do not meet VDOT design requirements. No southbound and northbound right lanes.

352.8



All Existing left turn lanes do not meet VDOT design 353.1 requirements. Ambulatory injury. No westbound left turn lane.

353.3

All existing turn lanes do meet the VDOT requirements.

Existing US 58 left turn lanes do not meet VDOT design requirements. Fatal injury.

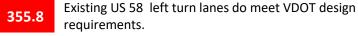
All exiting turn lanes do meet the VDOT requirements. 354.2 Visible injury.

Existing US 58 left turn lanes do meet VDOT design requirements.

All Existing US 58 turn lanes do meet VDOT design 355.1 requirements, except for WB right turn lane is short

Existing left turn lanes do meet VDOT design requirements.





356.2

- Existing US 58 left turn lanes do meet VDOT design requirements. Existing US 58 right turn lanes do meet VDOT design requirements.
- Existing US 58 left turn lanes do meet VDOT design requirements.
- Existing east bound US 58 left turn lanes do meet VDOT design requirements.

- Existing east bound US 58 left turn lanes do meet VDOT design requirements.
- **357.9** Existing US 58 left turn lanes are TWLTL turn lanes..
- Existing US 58 left turn lanes do not meet VDOT design requirements.

- Existing westbound US 58 left turn lanes do meet VDOT design requirements. (storage length).
- Existing US 58 left turn lanes do not meet VDOT design requirements. Fatal injury in short distance after the crossover
- Existing US 58 left turn lanes and eastbound right turn lane do meet VDOT design requirements. No westbound US 58 right lane. High crashes point in the intersection.



Existing US 58 left turn lanes and eastbound right turn lane do meet VDOT design requirements.

Existing US 58 eastbound right turn lane does not meet VDOT design requirements. High rate of crashes. Sight distance issue on eastbound approach.

Existing US 58 left turn lanes do meet VDOT design requirements. Existing US 58 eastbound right turn lane does not meet VDOT design requirements. (Ambulatory injury crash reported)

360.2

Existing westbound US 58 left turn lane do meet VDOT design requirements. ((Extra wide median))

Existing eastbound US 58 left turn lane and right turn lanes do not meet VDOT design requirements. High rate of crashes.

Existing US 58 left turn lanes do meet VDOT design requirements. Ambulatory injury.

All existing US 58 left turn lanes do meet VDOT design requirements.

Existing US 58 left turn lanes do not meet VDOT design requirements. Ambulatory crashes.

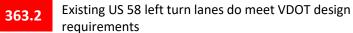
All existing US 58 left turn lanes do meet VDOT design requirements. . High crashes access point. Spacing does not meet VDOT requirements.

Existing US 58 left turn lanes do meet VDOT design requirements.

Existing US 58 left turn lanes do meet VDOT design requirements.

Existing US 58 left turn lanes do meet VDOT design requirements. No US 58 right turn lanes.



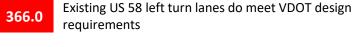


- All existing US 58 turn lanes do meet VDOT design requirements.
- Existing US 58 left turn lanes do not meet VDOT design requirements
- Existing westbound US 58 left turn lanes does not meet VDOT design requirements.

- Existing US 58 left turn lanes do meet VDOT design requirements. Visible injury.
- No US 58 left turn lanes.
- Existing US 58 left turn lanes do meet VDOT design requirements

- Existing US 58 right turn lanes do not meet VDOT design requirements. Sight issue to see the intersection due incline in both sides. High rate of visible injury crashes
- Existing US 58 left turn lanes do meet VDOT design requirements





Existing westbound US 58 left turn lane does not meet VDOT design requirements.

Existing US 58 left turn lanes do meet VDOT design requirements. Fatal crash

368.3

Existing US 58 left turn lanes do not meet VDOT design requirements

366.5

Existing US 58 left turn lanes do meet VDOT design requirements. Visible crash.

Existing US 58 left turn lanes do meet VDOT design requirements. Existing US 58 Westbound right turn lane does not meet VDOT design requirements.

Existing US 58 left turn lanes do not meet VDOT design requirements

Existing westbound US 58 left turn lane do meet VDOT design requirements

Existing eastbound US 58 left turn lane does not meet VDOT design requirements.

Existing US 58 left turn lanes do meet VDOT design requirements



Signalized Intersection

Unsignalized Intersection

Median Crossovers

Inadequate intersection spacing per VDOT standards

Existing eastbound US 58 left turn lanes do meet 369.2 VDOT design requirements

Existing eastbound US 58 left turn lane does meet 369.5 VDOT design requirements. Begin TWLTL east of intersection.

Existing US 58 TWLTL left turn lanes. No US 58 right 370.5 turn lanes. High rate of crashes.

Existing US 58 TWLTL left turn lanes. No US 58 right 370.9 turn lanes. High rate of crashes. TWLTL inconsistent.

371.3

Existing US 58 TWLTL left turn lanes. Driveways only.

Existing US 58 TWLTL left turn lanes. 369.8



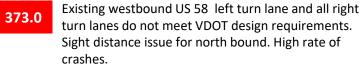
Existing US 58 left turn lanes TWLTL (inconsistent).

372.2

Existing US 58 left turn lanes TWLTL (inconsistent).
This intersection has overhead flashers. Westbound on-ramp from US 1 (Eastern) acceleration lane too short. Ambulatory crashes.

372.7 Westbound Acceleration lane is short.

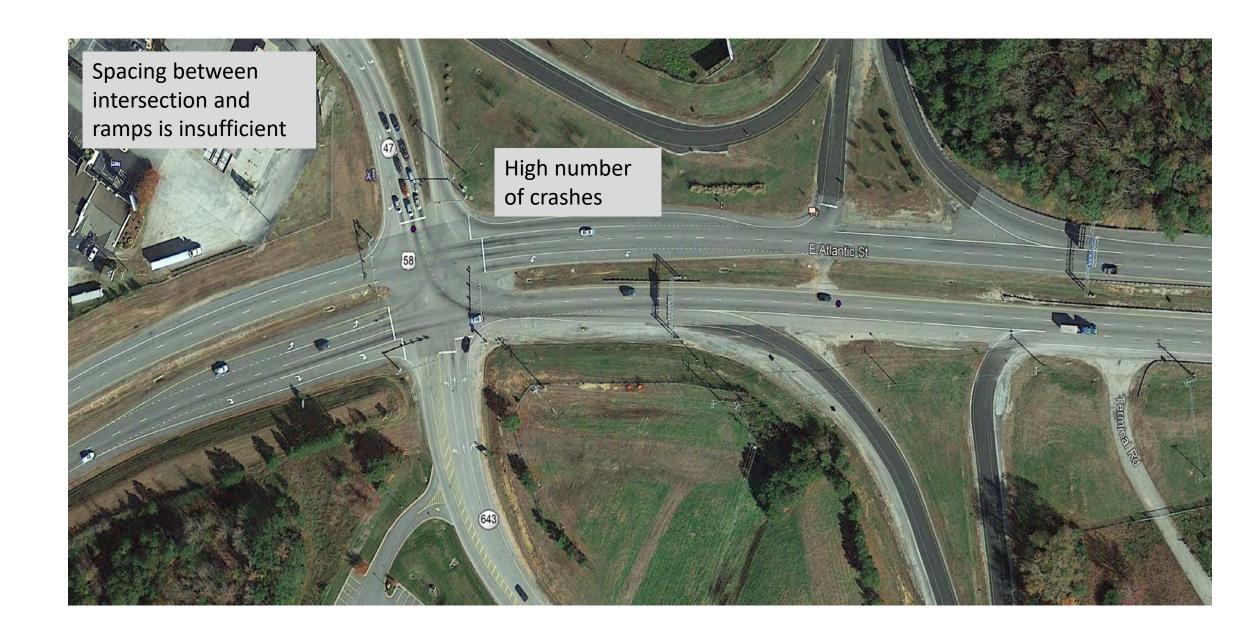


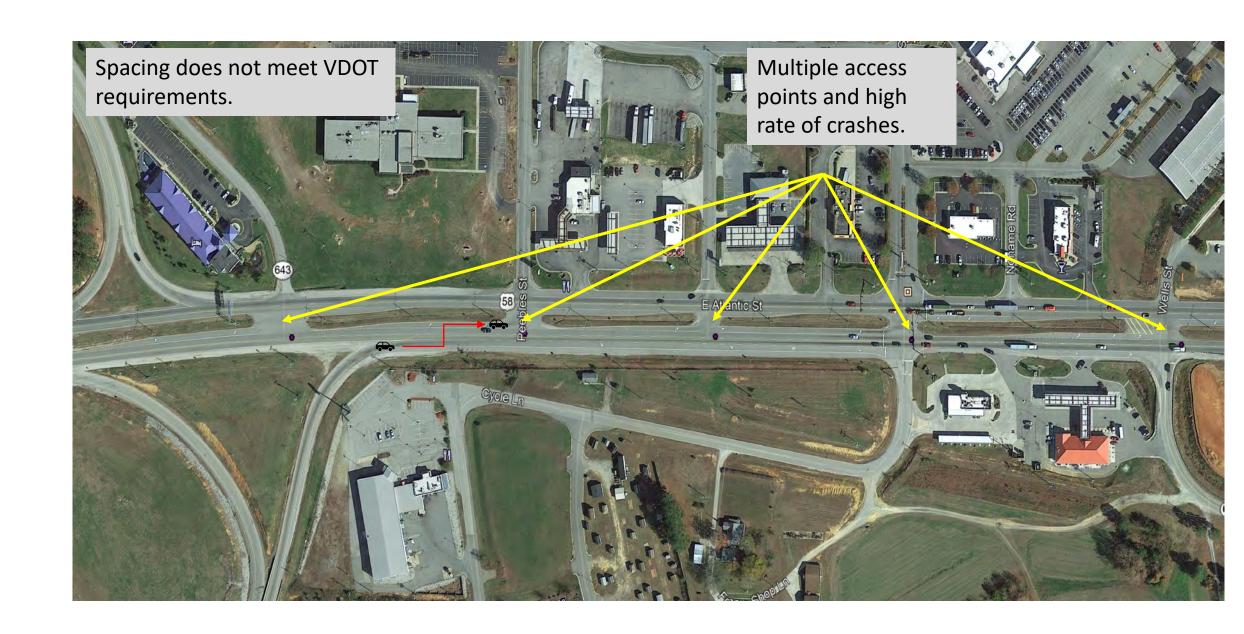


- Existing US 58 left turn lanes do meet VDOT design requirements
- Existing US 58 left turn lanes do meet VDOT design requirements. Existing eastbound US 58 right turn lane does not meet VDOT design requirements.
- Existing US 58 left turn lanes do not meet VDOT design requirements. Existing US 58 eastbound right turn lane does not meet VDOT design requirements. High rate of crashes.

- The intersection meets VDOT design requirements except for the distance between intersection and interchange ramp. High rate of visible injury crashes
- No US 58 left lanes. Fatal crashes.
- Existing US 58 left turn lanes do meet VDOT design requirements. No US 58 right turn lanes
- Existing US 58 left turn lanes do meet VDOT design requirements. Ambulatory injury crashes. Visible crash

- Existing US 58 left turn lanes do meet VDOT design requirements. Multiple ambulatory crashes. Visible crash.
- Existing US 58 left turn lanes do meet VDOT design requirements. Existing eastbound US 58 right turn lane does meet VDOT design requirements. Multiple access points. Multiple ambulatory crashes
- All exiting US 58 do meet the VDOT design requirements. Multiple access points. High rate of crashes.







Existing US 58 left turn lanes do meet VDOT design requirements.

377.4

All existing US 58 left and right turn lanes do not meet VDOT design requirements. ((Access points))

All Existing US 58 left and right turn lanes do meet VDOT design requirements.

379.3

Existing westbound US 58 right turn lane does not meet VDOT design requirements

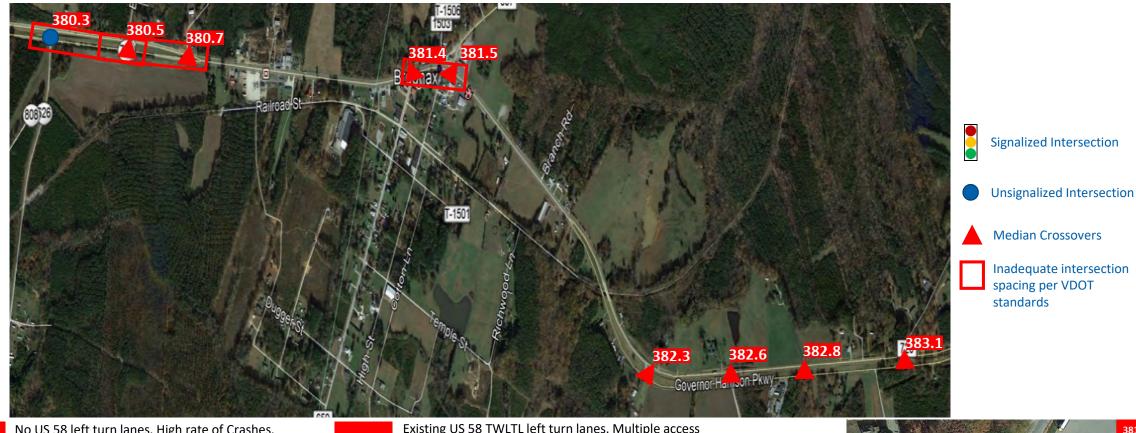
Existing US 58 left turn lanes do meet VDOT design requirements.

Existing US 58 left turn lanes do not meet VDOT design requirements. Existing eastbound US 58 right turn lane does not meet VDOT design requirements

Existing US 58 left turn lanes do meet VDOT design requirements.

All existing US 58 left turn lanes do meet VDOT design requirements.

Existing eastbound US 58 left turn lane does not meet VDOT design requirements. Fatal crash



No US 58 left turn lanes. High rate of Crashes.

Existing US 58 TWLTL left turn lanes. Multiple access points.

No US 58 left turn lanes exist.

No US 58 left lanes exist

No US 58 turn lanes exist. Is this for authorized vehicles? 382.6

No US 58 left lanes exist.

381.4

Existing US 58 TWLTL left turn lanes.

No US 58 left lanes exist

No US 58 left lanes exist.

382.8





383.4 No US 58 left turn lanes. Multiple driveways.

Existing US 58 westbound turn lane does not meet VDOT design requirements. No US 58 eastbound left turn lane. Fatal crash.

384.3 No US 58 left turn lanes

No US 58 Left turn lanes

Existing eastbound US 58 left turn lane does no meet VDOT requirements. No US 58 westbound left turn lane.((Access points))

385.4 No US 58 Left turn lane.

385.9 No US 58 Left turn lane.

Existing US 58 left turn lanes do not meet VDOT design requirements.

No US 58 Left turn lane. Ambulatory injury crashes in this access point.

No US 58 Left turn lane.

386.8



No US 58 left turn lanes.

Exiting US 58 eastbound left turn lane does not meet VDOT requirements. No US 58 westbound left turn lane. Exiting US 58 westbound right turn lane does not meet VDOT requirements. No US 58 eastbound right turn lane.

388.0

No US 58 left turn lanes. Visible crash.



390.9

No US 58 left turn lanes

Existing US 58 left turn lanes do not meet VDOT design requirements. No US 58 right turn lanes

391.2

391.3

391.0

Existing US 58 westbound left turn lane does not meet VDOT design requirements. No eastbound US 58 left turn lane

392.4

No US 58 left turn lanes

391.5 Existing US 58 eastbound left turn lane does not meet VDOT design requirements. No westbound US 58 left turn lane



394.5

394.8

396.3

No US 58 left turn lanes. . One Ambulatory injury crashes in this access point 394.5.

Exiting US 58 left turn lanes do not meet VDOT requirements. Exiting eastbound US 58 right turn lane does not meet VDOT requirements.

Existing westbound US 58 left turn lane does no meet VDOT requirements. No eastbound left turn lane.

Existing eastbound US 58 left turn lane does no meet VDOT requirements. No US 58 westbound left turn lane. Fatal crash

Existing westbound US 58 left turn lane does no meet VDOT requirements. No US 58 eastbound left turn lane

Existing westbound US 58 left turn lane does no meet VDOT requirements. No US 58 eastbound left turn lane

Existing westbound US 58 left turn lane does no meet VDOT requirements. No US 58 eastbound left turn lane.

Exiting US 58 left turn lanes do not meet VDOT requirements. No US 58 right turn lanes. No US 58 right turn lanes. Grade differential on US 58 eastbound and westbound. High rate of crashes.

398.0

Exiting US 58 left turn lanes do not meet VDOT requirements. No US 58 right turn lanes



No US 58 left turn lanes. High rate of crashes at access point 398.4

Exiting US 58 left turn lanes do not meet VDOT requirements.

399.6 No US 58 left turn lanes.

Existing westbound US 58 left turn lane does no meet VDOT requirements. No US 58 eastbound left turn lane

No US 58 left turn lanes.

Existing westbound US 58 left turn lane DOES meet VDOT requirements. No US 58 eastbound left turn lane. Existing eastbound US 58 right turn lane DOES meet VDOT requirements.

Existing eastbound US 58 left turn lane does meet VDOT requirements. No US 58 westbound left turn lane

No US 58 left turn lanes.



Exiting US 58 left turn lanes do not meet VDOT requirements. No US 58 right turn lanes.

404.4 No US 58 left turn lanes.

403.0 No US 58 left turn lanes.

O1..1 Exiting US 58 left turn lanes do not meet VDOT requirements.

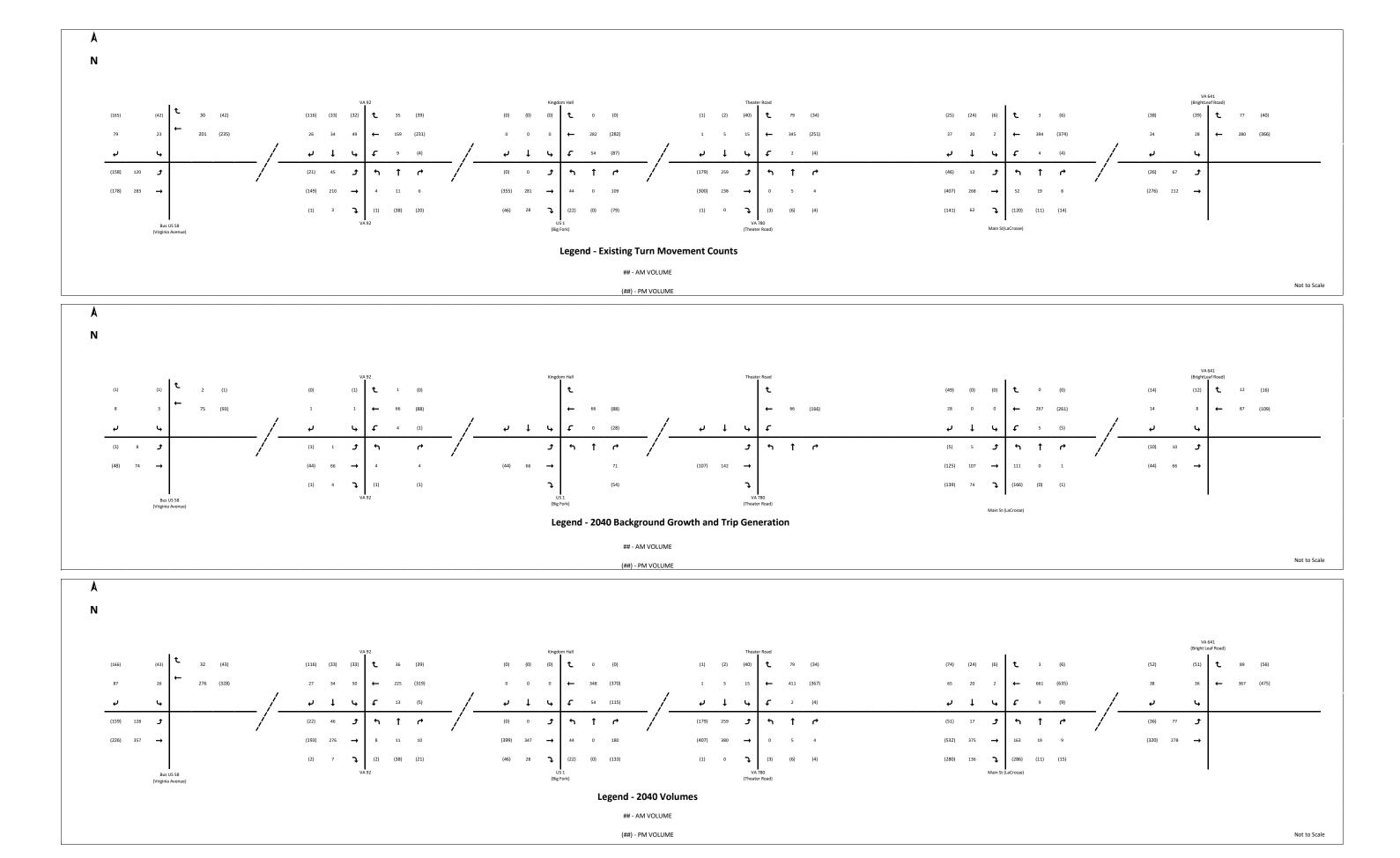
403.4 No US 58 left turn lanes.

403.9 No US 58 left turn lanes.

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APPENDIX D: US 58 INTERSECTION VOLUMES

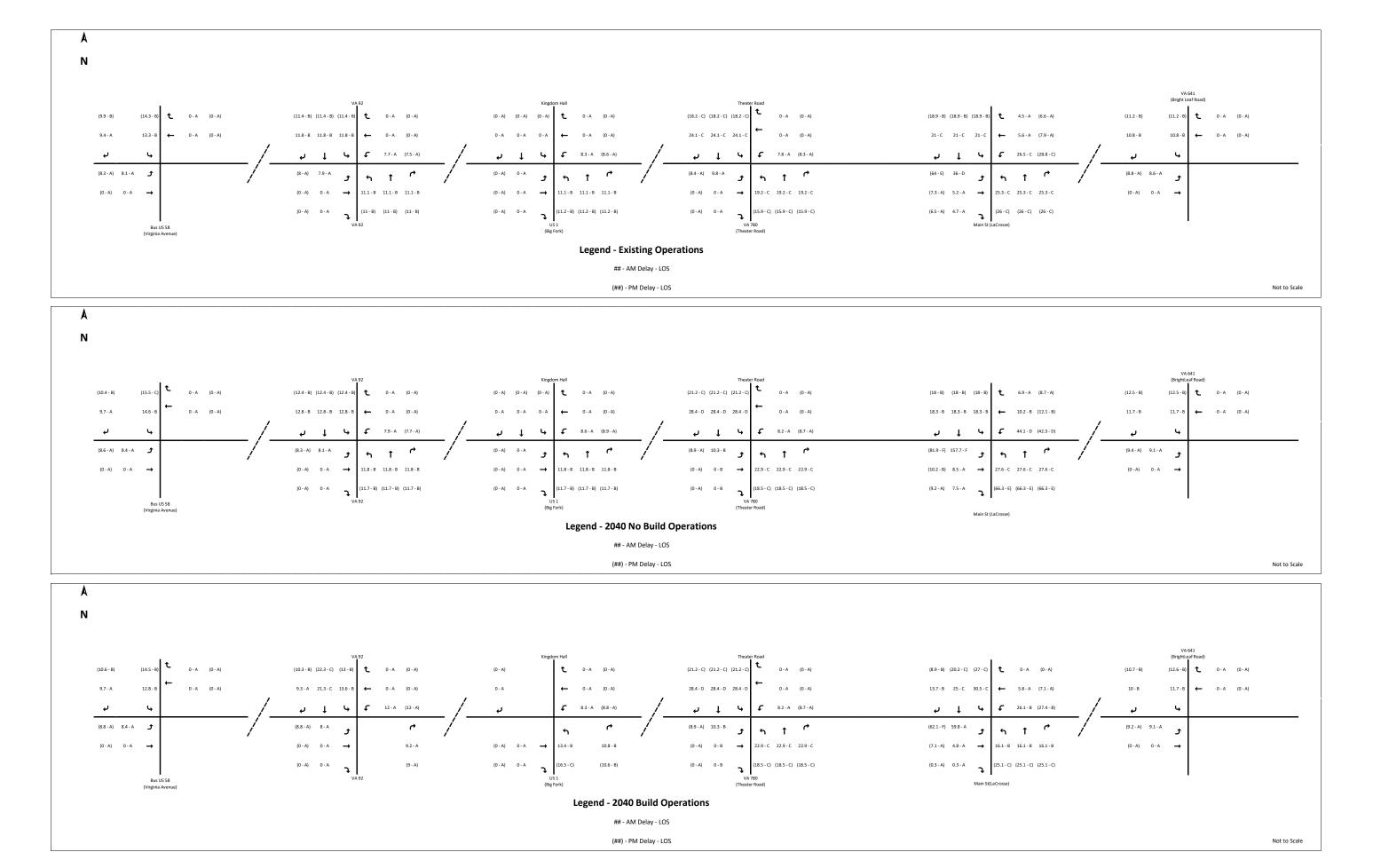
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APPENDIX E: US 58 INTERSECTION OPERATIONS

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APPENDIX F: TOWN OF SOUTH HILL MEMORANDUM



Memorandum

TO: Christopher Detmer, VDOT **DATE:** October 8, 2019

FROM: Daniel Scolese, P.E. SUBJECT: Town of South US 58 Corridor Study

The purpose of this memorandum is to present the results and recommendations for the additional analyses conducted to evaluate alternatives for the US 58 Arterial Preservation Plan within the Town of South Hill. The study area is focused between the intersection of US 58 and Maple Lane and the intersection of US 58 and High Street. An initial study was conducted in 2018, that focused on improving the safety between the I-85 northbound off-ramp onto US 58 eastbound as well as evaluating three intersection improvements within the Town of South Hill. The goals of this follow-up study within the Town of South Hill are to:

- Improve the safety of US 58;
- Improve and maintain the capacity of US 58; and
- Incorporate and support the Town of South Hill's Economic Development goals.

Traffic counts and the existing conditions analysis from the 2018 study were carried forward to this follow-up study. A detailed crash history is provided at the end of this memo that highlights the significant safety concerns within the study area. Alternative designs were analyzed and reviewed in meetings with the Town of South Hill on March 27, 2019, May 20, 2019 and June 20, 2019. The recommendations were presented to the Town of South Hill Council on July 31, 2019 and adopted by the Town Council on August 12, 2019. The final recommendations are attached and are a result from these forums.

Future Volumes

Future turn movements volumes were calculated using a background rate of one percent, trip generation for potential development along the corridor between Mecklenburg and Brunswick county, and the potential economic growth within the Town of South Hill. The US 58 Richmond Arterial Preservation Plan Report includes further discussion on the development of the future traffic volumes. The future land use and development within the Town of South Hill was determined using existing documentation as well as input from VDOT and the Town of South Hill. The assumed land uses can be found attached at the end of this memo. Future traffic volumes were developed for the following scenarios:

- 2040 No-Development within Town of South Hill: No Build Volumes;
- 2040 No-Development within Town of South Hill: Build Volumes;
- 2040 Development occurring within Town of South Hill: No Build Volumes; and
- 2040 Development occurring within Town of South Hill: Build Volumes.

Future Recommendations and Operations:

The final adopted recommendations for the corridor are:

Intersection of US 58 with Maple Lane

• Construct right-turn lanes on eastbound and westbound Maple Lane. Traffic conditions at this location should be monitored into the future to determine if any additional improvements are needed.

Intersection of US 58 with Country Lane

- Reconfigure the intersection to reduce traffic signal phasing by relocating the US 58 left-turn movements and southbound thru-movements from Country Lane.
- Reconstruct the westbound US 58 right-turn lane onto US BUS 58 as a continuous right-turn.

US 58 and I-85 Interchange

- Reconstruct the interchange as either a Diverging Diamond Interchange (DDI) or Roundabouts configuration.
- Conduct an Interchange Modification Report (IMR) for approval from FHWA and VDOT.

US 58 and Thompson Street

• Reconfigure intersection to right-in/right-out and re-route movements through interparcel connections between Thompson Street and Peebles Street.

US 58 and Peebles Street

Maintain access and lengthen eastbound left-turn lane as determined by a traffic capacity analysis. As
development occurs, additional improvements will be required and final determination of appropriate
traffic control shall be determined through a traffic signal warrant analysis, signal justification report, and
approvals by District, State, and Federal officials.

US 58 and Crowder Street

Reconfigure intersection to right-in/right-out.

US 58 and Cycle Lane

• Reconfigure the intersection to reduce traffic signal phasing by relocating eastbound and westbound leftturn movements on US 58 and northbound and southbound thru-movements from Cycle Lane.

US 58 and High Street

Reconstruct the intersection to a roundabout. As development occurs, the northbound approach on High
Street may need to be reconfigured to permit only right-turn movements to maintain the capacity of the
intersection. The northbound left-turns and through movements will use the Cylce Lane traffic signal via
the interparcel connection between Cycle Lane and High Street.

Detailed configuration concepts and operational results are attached to this memo. Table 1 summarizes the delay and LOS for the US 58 at-grade intersections. Table 2 and Table 3 summarize the delay and travel times for the US 58 and I-85 interchange. It should be noted that the diverging diamond traffic signals are coordinated so that vehicles stop only once at a traffic light. Figures of the delay and LOS results are attached to this memo.

Intersection	Scenario	Overall Delay					Delay per La	ane Group l (Level of		ch (sec/veh	n)			
		(LOS)		Eastbound			Westboun			Northboun			Southbound	
			LT	TH	RT	LT	TH AM Peak	RT	LT	TH	RT	LT	TH	RT
			8.1	0.0	0.0	7.9	0.0	0.0	12.7	11.1	11.1	11.7	11.7	11.7
	2018 Existing	2.0	Α	Α	Α	Α	Α	Α	В	В	В	В	В	В
		Α		1.1 (A)			0.3 (A)			11.1 (B)			11.7 (B)	
	2040 No	2.3	8.0	0.0	0.0	8.2	0.0	0.0	14.6	14.6	9.7	13.2	12.0	12.0
	Development No Build	Α	В	A 0.8 (A)	Α	Α	0.6 (A)	Α	В	B 11.9 (B)	Α	В	12.0 (B)	Α
	2040 No		8.0	0.0	0.0	8.2	0.0	0.0	14.6	14.6	9.7	13.2	12.0	12.0
	Development	2.3	В	Α	A	A	A	A	В	В	A	В	Α	А
	Build	Α		0.8 (A)			0.6 (A)			11.9 (B)			12.0 (B)	
	2040	8.7	8.3	0.0	0.0	8.7	0.0	0.0	27.4	27.4	9.9	19.2	42.2	42.2
	Development No Build	A	Α	0.7 (A)	Α	Α	2.0 (A)	Α	D	D 17.3 (C)	Α	С	42.0 (E)	E
	2040		8.3	0.7 (A)	0.0	8.7	0.0	0.0	27.4	27.4	9.9	19.2	42.0 (L)	42.2
	Development	8.7	A	А	А	A	A	A	D	D	А	С	E	E
Maple Ln & US 58	Build	Α		0.7 (A)			2.0 (A)			17.3 (C)			42.0 (E)	
Wapic Lif & 03 30							PM Peak							
	2018 Existing	1.9	7.7 A	0.0 A	0.0	8.8 A	0.0 A	0.0 A	11.5 B	9.5 A	9.5 A	11.5 B	11.5 B	11.5 B
	2016 EXISTING	А	А	0.7 (A)	Α	A	0.2 (A)	I A	В	9.6 (A)	А	В	11.5 (B)	В
	2040 No		7.6	0.0	0.0	9.1	0.0	0.0	13.0	13.0	9.7	11.7	9.4	9.4
	Development	1.9	Α	Α	Α	Α	Α	Α	В	В	Α	В	Α	Α
	No Build	Α		0.6 (A)			0.4 (A)			11.6 (B)			9.6 (A)	
	2040 No	1.9	7.6	0.0	0.0	9.1	0.0	0.0	13.0	13.0	9.7	11.7	9.4	9.4
	Development Build	А	Α	0.6 (A)	Α	А	0.4 (A)	Α	В	11.6 (B)	Α	В	9.6 (A)	Α
	2040		8.0	0.0	0.0	9.8	0.0	0.0	22.3	22.3	11.5	22.0	10.3	10.3
	Development	5.3	Α	Α	Α	Α	Α	Α	С	С	В	С	В	В
	No Build	Α		0.5 (A)			1.5 (A)			16.0 (C)			11.0 (B)	
	2040	5.3	8.0	0.0	0.0	9.8	0.0	0.0	22.3	22.3	11.5	22.0	10.3	10.3
	Development Build	А	Α	0.5 (A)	Α	А	1.5 (A)	А	С	16.0 (C)	В	С	11.0 (B)	В
	Bullu	A		0.5 (A)			AM Peak	Hour		10.0 (C)			11.0 (b)	
		22.4	40.3	17.0	15.8	40.5	17.6	14.3	45.4	37.9	38.4	36.7	26.8	27.1
	2018 Existing		D	В	В	D	В	В	D	D	D	D	С	D
		С		19.4 (B)			16.6 (B)			38.8 (D)			35.2 (D)	10.0
	2040 No Development	23.4	46.3 D	23.3 C	15.7 B	60.0 E	22.7 C	14.1 B	36.2 D	17.0 B	16.7 B	34.7 C	10.9 B	10.9 B
	No Build	С	U	24.8 (C)	В		19.0 (B)	В	U	18.1 (B)	В	L C	31.0 (C)	В
	2040 No		NA	17.1	0.1	33.0	17.0	0.5	25.4	26.3	25.4	20.0	33.2	17.2
	Development	13.4		В	Α	D	В	Α	С	С	С	В	D	В
	Build	В		15.9 (B)			7.9 (A)			25.8 (C)			20.7 (C)	
	2040 Development	24.6	46.3	24.2 C	15.8 B	60.0	27.4 C	14.5 B	36.2 D	17.0 B	16.7 B	35.5 D	10.8 B	10.8 B
	No Build	С	D	25.5 (C)	Б	E	21.7 (C)	ь	U	18.1 (B)	Б	, b	31.8 (C)	В
	2040		NA	18.3	0.0	36.3	20.3	0.7	25.4	26.3	25.4	19.3	32.5	16.5
	Development	14.4		В	Α	D	С	Α	С	С	С	В	D	В
Country Ln & US 58	Build	В		16.6 (B)			10.9 (B)			25.8 (C)			20.0 (C)	
•			53.7	23.9	22.2	55.8	PM Peak 23.9	15.9	55.9	52.1	57.5	43.2	27.1	26.8
	2018 Existing	32.3	55.7 D	23.9 C	22.2 C	55.8 E	23.9 C	15.9 B	55.9 E	52.1 D	57.5 E	43.2 D	27.1 C	26.8 C
		С		27.6 (C)			17.6 (B)			55.1 (E)			41.1 (D)	
	2040 No	25.4	45.8	26.8	21.7	77.3	26.6	13.3	43.3	24.6	23.3	34.9	10.7	10.3
	Development		D	C (c)	В	E	C	В	D	C C	С	С	B	В
	No Build 2040 No	С	NA	28.7 (C) 16.8	0.1	31.8	16.7 (B) 15.8	1.0	26.9	25.4 (C) 30.5	26.8	25.3	31.9 (C) 32.8	16.8
	Development	16.3	INA	10.8 B	0.1 A	D D	15.8 B	1.0 A	26.9 C	30.5 C	26.8 C	25.3 C	32.8 D	10.8 B
	Build	В		15.0 (B)			4.1 (A)		Ü	28.5 (C)			25.5 (C)	
	2040	30.0	45.8	32.2	21.7	77.3	30.4	15.2	43.3	25.0	23.7	43.8	10.7	10.3
	Development		D	С	С	E	С	В	D	С	С	D	В	В
	No Build	С	8/ 6	32.9 (C)	0.1	20.7	20.4 (C)	1.5	22.6	25.8 (C)	47.0	20.2	40.2 (D)	47.0
	2040 Development	19.1	NA	26.4 C	0.1 A	39.7 D	23.7 C	1.5 A	23.6 C	25.4 C	17.2 B	28.2 C	33.3 D	17.3 B
	Build	В		23.9 (C)			21.3 (C)			21.2 (C)	U		28.1 (C)	
				n of Cou									(0)	

Table 1: Town of South Hill US 58 At-Grade Intersection Operations

Table 2: Town of South Hill US 58 At-Grade Intersection Operations (Cont.)

Marie			Overall			Delay per Lane Group b				by Approa	ch (sec/veh	1)			
Control of State S	Intersection	Scenario													
STEEL STORE			(LOS)	LT	TH	RT	LT			LT	TH	RT	LT	TH	RT
2015 Footbase State St				0.5	0.0	NΛ	NA						12.0	NΑ	12.0
2004 No. 3.0		2018 Existing	0.5			INA	INA				NA			INA	
Development A		2010 Existing	Α											12.8 (B)	
Development A		2040 No	0.4	8.5	0.0	NA	NA	0.0	0.0				9.8	NA	9.8
200 No Development		Development	0.4	Α		NA			Α		NA		Α		Α
Development No Build No Bui			Α												
Suid A O O O O O O O O O			0.2	NA		NA	NA				NI A		NA	NA	
2040 2040		·	۸						А		NA			10.1 (B)	В
Development No Build				9.4		NA	NA		0.0				10.7		10.7
No build 2040 O.1			0.3	-							NA			.,,,	
Crowder St & U.5 \(\) S			Α											10.7 (B)	l.
Crowder St & US 58 Section Sec		2040	0.1	NA	0.0	NA	NA	0.0	0.0				NA	NA	10.1
PMP Peak Hour Pack Hour			0.1						Α		NA				В
2013 Existing	Crowder St & US 58	Build	Α		0 (A)									10.1 (B)	
2018 Existing				0.0	0.0								447		447
2040 No Development No Build A D. Development Develo		2018 Existing	1.0			NA	NA NA				NΔ			NA	
2040 No Development No Build A		2010 LAISTING	Α								IVA			14.7 (B)	ь
Development No Build		2040 No		8.8	1	NA	NA		0.0				10.4		10.4
2040 No Development Build A 0,0 NA NA 0,0 NA NA 0,0 NA NA 0,0 NA NA NA 11.3 B 11.3 (B)			0.7								NA				
Development		No Build	Α		0.6 (A)			0.0 (A)						10.4 (B)	
Development Build A		2040 No	0.4	NA	0.0	NA	NA	0.0	0.0				NA	NA	10.3
2040 Development No Build A		·							NA					В	
Development No Build A			Α												
No Build			0.6				NA				NIA			NA	
2040 Development Build A			۸	В		NA			А		NA		В	12.2 (B)	В
Development Build A				NΔ		NΔ	NΔ		0.0				NΔ		11 3
Build A			0.3	""		1473	10,0				NA		10,	1473	
2018 Existing 13.9 34.0 9.4 3.7 37.3 13.3 7.1 33.6 33.6 32.6 31.5 31.5 30.9			Α		•			-						11.3 (B)	ı
2018 Existing 2018 Existing 2018 Existing 2040 No Development No Build No Build A 2.1 (A) A D B A C C C C C C C C C															
2018 Existing C A A D B A C C C C C C C C C		2018 Existing	13.9	34.0	9.4	3.7	37.3	13.3	7.1	33.6	33.6	32.6	31.5	31.5	30.9
2040 No Development No Build Development No Build A				С		Α	D		Α	С		С	С		С
Development No Build B		221211	В	26.0			42.2			20.2		20.0	27.0		25.7
No Build B			14.8												
2040 No Development Build A Development Build A Development Build A Development Build A Development No Build A Development Build A Development A A A A A A A A A			R	U		A	D		В	C		C	U		D
Development Build				NA		0.0	NA		0.1	26.4		25.2	27.5		25.3
Cycle Ln & US58 Part			4.0												
Development No Build C C C C D D D D D D		Build	Α		2.1 (A)			1.8 (A)			26.3 (C)			26.6 (C)	
Development No Build C D B B D C B C C C D D D D D D D		2040	23.8	41.2	19.7	15.8	41.1	22.1	14.7	34.5	34.5	29.1	41.1	41.1	38.4
Cycle Ln & US 58 2040 Development Build 6.9 Development Build NA 5.4 A A A A A A A A A A A A A A A A A A A				D		В	D		В	С		С	D		D
Cycle Ln & US 58 Development Build 6.9 A A A A A A B			С							10.0		10.0			
Build A 4.4 (A) 5.0 (A) 17.7 (B) 14.2 (B)			6.9	NA			NA NA				NA			NA	
18.3 36.9 15.8 5.7 54.7 17.5 7.8 31.5 30.3 29.1 29.1 26.8		· ·	Δ			A			А	В	17 7 (B)	В	В	14 2 (R)	В
2018 Existing B B A D B A C C C C C C C C C	Cycle Ln & US 58	Duna			(/,/				Hour		2717 (5)			1 (5)	
D B A D B A C C C C C C C C C			10.2	36.9	15.8	5.7	54.7	17.5	7.8	31.5	31.5	30.3	29.1	29.1	26.8
2040 No Development No Build 25.6 42.4 22.3 17.0 77.3 24.6 20.3 27.6 27.6 26.2 39.4 39.4 33.6 33.6 20.0 20		2018 Existing	18.3	D	В	Α	D	В	Α	С		С	С	С	С
Development No Build C 22.7 (C) 22.5 (C) 25.0 (C) 27.3 (C) 36.7 (D) C			В												ı
No Build C 22.7 (C) 25.0 (C) 27.3 (C) 36.7 (D) 2040 No Development Build 6.9 NA 4.9 0.1 NA 4.3 0.2 20.9 NA 20.0 23.2 NA 20.3 Build A 4.1 (A) 3.2 (A) 20.7 (C) 22.0 (C) 22.0 (C) 2040 31.3 D C C D C B D D C D D C No Build C 27.7 (C) 30.2 (C) 41.5 (D) 36.7 (D) 36.7 (D) 2040 NA 10.6 NA 10.3 0.3 NA 9.8 0.2 22.3 NA 13.8 NA 13.8 Development Build B A A A A C B B B B B B B A A A C B B B B B B B B B </td <td></td> <td></td> <td>25.6</td> <td></td>			25.6												
2040 No Development Build A 4.9 0.1 NA 4.3 0.2 20.9 NA 20.0 23.2 NA 20.3			•	D		В	E		С	С		С	D		С
Development Build 6.9 A A A A A A C B C C C 2040 Development No Build 31.3 D 42.4 D 29.0 D 22.0 D 45.4 D 30.3 D 20.3 D 44.7 D 44.7 D 26.6 D 39.4 D 39.4 D 33.6 D 33.6 D C D C D C D C D C D C D C D C D C D C D C D C D C D C D C D C D C D C D C D D C D D C D D C D D C D D C D D C D D C D NA 13.8 D NA NA P NA A A A A A A A			L	NΙΛ		0.1	NA		0.2	20.0		20.0	22.2		20.2
Build A 4.1 (A) 3.2 (A) 20.7 (C) 22.0 (C) 2040 Development No Build 31.3 42.4 29.0 22.0 45.4 30.3 20.3 44.7 44.7 26.6 39.4 39.4 33.6 Development No Build C C C D C B D D C D C 2040 Development Build NA 10.3 0.3 NA 9.8 0.2 22.3 NA 13.5 15.0 NA 13.8 B A A A A A C B B B B T,3 (A) T,8 (A) 21.1 (C) 14.5 (B)			6.9	IVA			INA				IVA			INA	
2040 Development No Build C 27.7 (C) 30.2 (C) 45.4 (C) 30.3 (C) 41.5 (D) 36.7 (D) 2040 Development Build B 7.3 (A) 7.8 (A) 20.0 44.7 (A4.7 (B.6 (C) 39.4 (A7.8) 33.6 (C) 39.4 (A7.8) 33.6 (C) 44.7 (C) 47.7 (C) 4		· ·	Α								20.7 (C)		Ť	22.0 (C)	
Development No Build 31.3 D C C D C B D D C D C 2040 Development Build 10.6 NA 10.3 0.3 NA 9.8 0.2 22.3 NA 13.5 15.0 NA 13.8 B A A A A C B B B B B 7.3 (A) 7.8 (A) 21.1 (C) 14.5 (B)				42.4		22.0	45.4		20.3	44.7		26.6	39.4		33.6
2040 Development Build B 7.3 (A) NA 9.8 0.2 22.3 NA 13.5 15.0 NA 13.8 B A A A A C B B B B B B B B B B B B B B		Development	31.3	D	С			С					D		
Development B B A A A A C B B B B Build B 7.3 (A) 7.8 (A) 21.1 (C) 14.5 (B)			С												
Development B A A A C B B B B Build B 7.3 (A) 7.8 (A) 21.1 (C) 14.5 (B)			10.6	NA			NA				NA			NA	
		· ·				Α			Α	С		В	В		В
Table 3: Town of South Hill US 58 At-Grade Intersection Operations (Cont.)														14.5 (B)	

Table 3: Town of South Hill US 58 At-Grade Intersection Operations (Cont.)

		Overall												
Intersection	Scenario	Delay	Eastbound		Westbound			Northboun	d	Southbound				
		(LOS)	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
							AM Peak	Hour						
		1.3	8.9	0.0	0.0	8.1	0.0	0.0	20.5	20.5	20.5	10.4	10.4	10.4
	2018 Existing	1.5	Α	Α	Α	Α	Α	Α	С	С	С	В	В	В
		Α		0.3 (C)			0.1 (A)			20.5 (C)			10.4 (B)	
	2040 No	0.9	8.8	0.0	0.0	8.4	0.0	0.0	15.5	15.5	15.5	10.3	10.3	10.3
	Development	0.5	Α	Α	Α	В	Α	Α	С	С	С	В	В	В
	No Build	Α		0.3 (A)			0.1 (A)			15.5 (C)			10.3 (B)	
	2040 No	4.5	13.5	3.1	3.2	11.3	3.8	0.0	13.5	0.0	6.0	0.0	0.0	0.0
	Development	4.5	В	Α	Α	В	Α	Α	В	Α	Α	Α	Α	Α
	Build	Α		4.3 (A)			4.0 (A)			12.4 (B)			0.0 (A)	
	2040	2.6	10.1	0.0	0.0	8.7	0.0	0.0	27.4	27.4	27.4	11.7	11.7	11.7
	Development	2.0	В	Α	Α	Α	Α	Α	D	D	D	В	В	В
	No Build	Α		0.3 (A)			0.9 (A)			27.4 (D)			11.7 (B)	
	2040	5.1	14.7	4.3	3.8	11.1	3.6	0.0	NA	NA	2.7	0.0	0.0	0.0
	Development		В	Α	Α	В	Α	Α			Α	Α	Α	Α
High St & US 58	Build	Α		5.4 (A)			5.0 (A)			2.7 (A)			0.0 (A)	
riigii St & OS SO	PM Peak Hour													
		3.2	8.9	0.0	0.0	9.1	0.0	0.0	46.7	46.7	46.7	15.1	15.1	15.1
	2018 Existing	3.2	Α	Α	Α	Α	Α	Α	E	E	Е	С	В	В
		Α		0.3 (C)			0.1 (A)			46.7 (E)			15.1 (C)	
	2040 No	1.3	8.9	0.0	0.0	9.0	0.0	0.0	17.9	17.9	17.9	11.2	11.2	11.2
	Development	2.0	Α	Α	Α	В	Α	Α	С	С	С	В	В	В
	No Build	Α		0.6 (A)			0.3 (A)	1		17.9 (C)			11.2 (B)	
	2040 No	4.6	13.7	3.2	3.3	11.5	3.9	3.9	13.7	0.0	6.2	0.0	0.0	0.3
	Development		В	Α	Α	В	Α	Α	В	Α	Α	Α	Α	Α
	Build	Α		4.3 (A)	1		4.0 (A)	1		12.4 (B)			0.3 (A)	
	2040	13.8	10.3	0.0	0.0	10.7	0.0	0.0	107.5	107.5	107.5	18.0	18.0	18.0
	Development		В	Α	Α	В	Α	Α	F	F	F	С	С	С
	No Build	В		0.6 (A)			2.0 (A)			107.5 (F)			18.0 (C)	
	2040	5.4	15.5	4.7	4.3	11.3	3.7	3.7	NA	NA	2.7	0.0	0.0	0.3
	Development	-	В	Α	Α	В	Α	Α			Α	Α	Α	Α
	Build	Α		5.7 (A)			5.7 (A)			2.7 (A)			0.3 (A)	

Table 4: Town of South Hill US 58 At-Grade Intersection Operations (Cont.)

	Scenario	Overall					Delay per La							
Intersection		Delay		Eastbound			Westbound			Northboun			Southboun	
		(LOS)	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
	AM Peak Hour													
	2040 No Build	See Travel Times												
	2040	4.2	NA	3.8	2.9	NA	3.1	3.0				14.0	6.3	6.0
	Roundabout	4.2		Α	Α		Α	Α		NA		В	Α	Α
	anaabaat	Α		3.7 (A)			3.0 (A)						9.2 (A)	
		18.3	NA	22.7	0.0	NA	18.2	NA				11.2	NA	18.2
	2040 DDI			C	Α		B			NA		В	440 (D)	В
I-85 Southbound & US		В		20.1 (C)			18.2 (B)	lla					14.9 (B)	
58							PM Peak I	nour						
	2040 No Build						See	e Travel Tir	mes					
	2040 Roundabout	4.0	NA	4.2	2.9	NA	3.0	3.0				14.4	6.7	6.4
		4.0		Α	Α		Α	Α	NA			В	Α	Α
		Α		4.0 (A)	1		3.0 (A)						9.3 (A)	
		12.8	NA	24.2	0.0	NA	27.5	NA				13.3	NA	16.1
	2040 DDI			C	Α		C			NA		В		В
	B 20.3 (C) 27.5 (C)											14.9 (B)		
	AM Peak Hour													
	2040 No Build	See Travel Times												
	2040	4.9	10.6	3.0	NA	NA	4.9	3.0	13.5	5.2	3.1			
	Roundabout	7.5	В	Α			Α	Α	В	Α	Α	1	NA	
		Α		3.8 (A)			4.6 (A)			8.6 (A)				
		17.9	NA	23.6	NA	NA	17.6	0.0	17.9	NA	11.5		A1.A	
I-85 Northbound & US	2040 DDI	В		23.6 (C)			В	Α	В		В		NA	
1-85 Northbound & US 58		В		23.0 (C)			PM Peak I	Hour						
30							vi i cak i							
	2040 No Build						See	e Travel Tir	nes					
	2040	4.2	10.6	2.9	NA	NA	4.4	3.0	15.6	7.3	4.3			
	Roundabout		В	Α			Α	Α	С	Α	Α	1	NA	
		Α		3.4 (A)			4.2 (A)			8.4 (A)				
		22.2	NA	25.2	NA	NA	26.6	0.1	15.6	NA	14.9			
	2040 DDI	6		C 25 2 (6)			C 22.2 (C)	Α	В	45.2 (5)	В		NA	
		С		25.2 (C)			22.3 (C)		tions	15.3 (B)				

Table 5: US 58 & I-85 Interchange Operations

Scenario	US 58 & I-85 Interchange Travel Times (sec)									
	Eastbound	Westbound								
AM Peak Hour										
2040 No Build	49	53								
2040 Roundabout	47	45								
2040 DDI	34	35								
PM Peak Hour										
2040 No Build	55	59								
2040 Roundabout	48	54								
2040 DDI	34	34								

Table 6: US 58 & I-85 Interchange Travel Times

Operationally, the recommendations improved delay in the AM and PM Peak Hours. Travel times in 2040 for both conditions improved as well.

The recommendations were also developed to reduce crashes. The following summarizes the anticipated reduction in crashes:

- Intersection of US 58 and Country Lane: Decreases crashes up to 25%
- US 58 and I-85 Interchange: DDI expected to decrease crashes up to 30% and Roundabouts would decrease crashes up to 20%
- Between Thompson Lane and High Street on US 58: Decreases crashes up to 40%

In addition to the benefits of reduced delay and improved safety, the recommendations support the Town of South Hill's economic development efforts by providing the additional capacity on US 58 and intersecting roadways. It is important to note that each of the recommendations can be constructed independently. This flexibility allows for separate project submissions by the Town of South Hill and phasing of construction. The preferred recommendations are provided following this page containing detailed information, opinion of costs, and concepts.

Attachments:

US 58 Town of South Hill Study Area

US 58 Crash History

2018 Existing Turn Movement Counts

US 58 Town of South Hill Land Use

2040 No-Development within Town of South Hill: No Build Volumes

2040 No-Development within Town of South Hill: Build Volumes

2040 Development occurring within Town of South Hill: No Build Volumes

2040 Development occurring within Town of South Hill: Build Volumes

2018 Existing Operations

2040 No-Development within Town of South Hill: No Build Operations

2040 No-Development within Town of South Hill: Build Operations

2040 Development occurring within Town of South Hill: No Build Operations

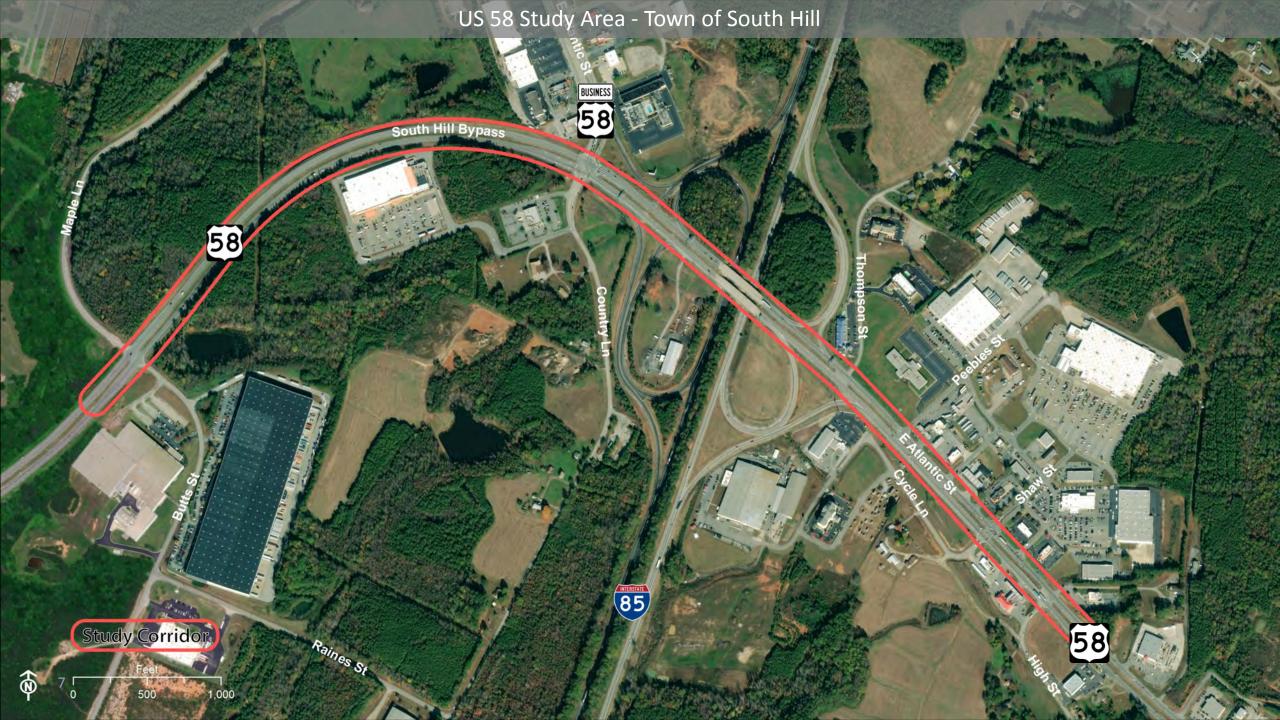
2040 Development occurring within Town of South Hill: Build Operations

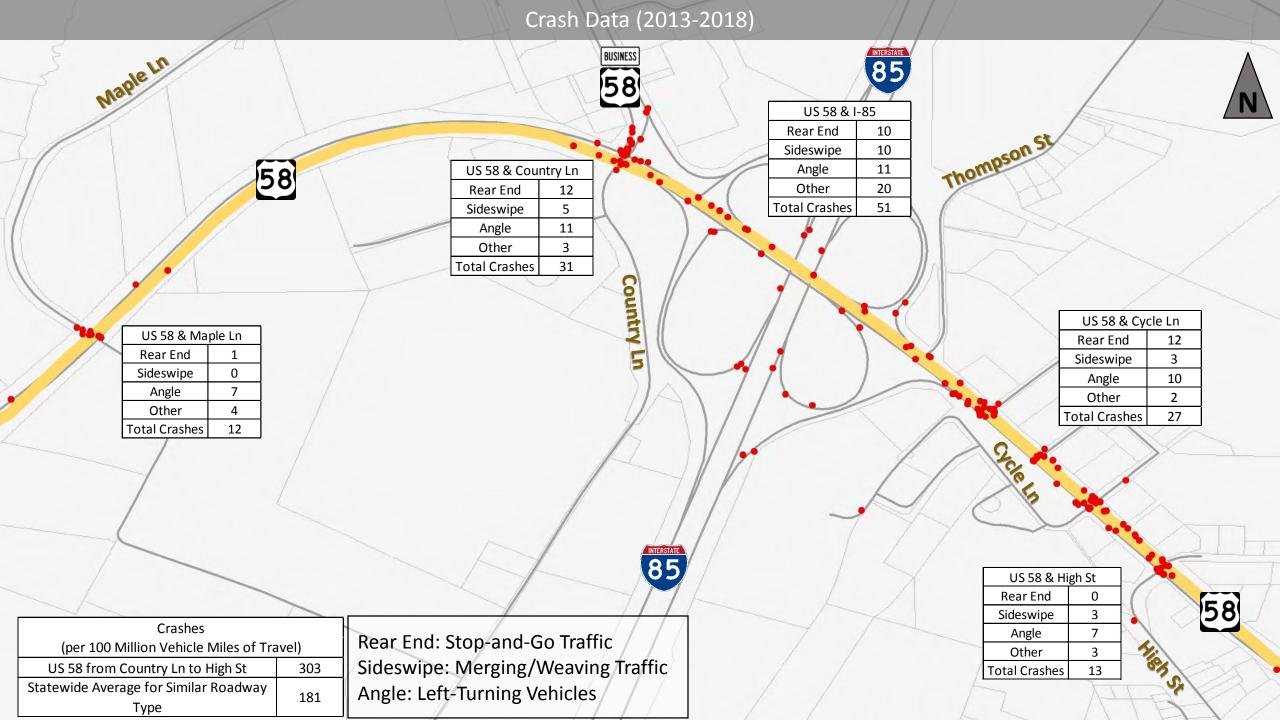
Concepts of Recommendations:

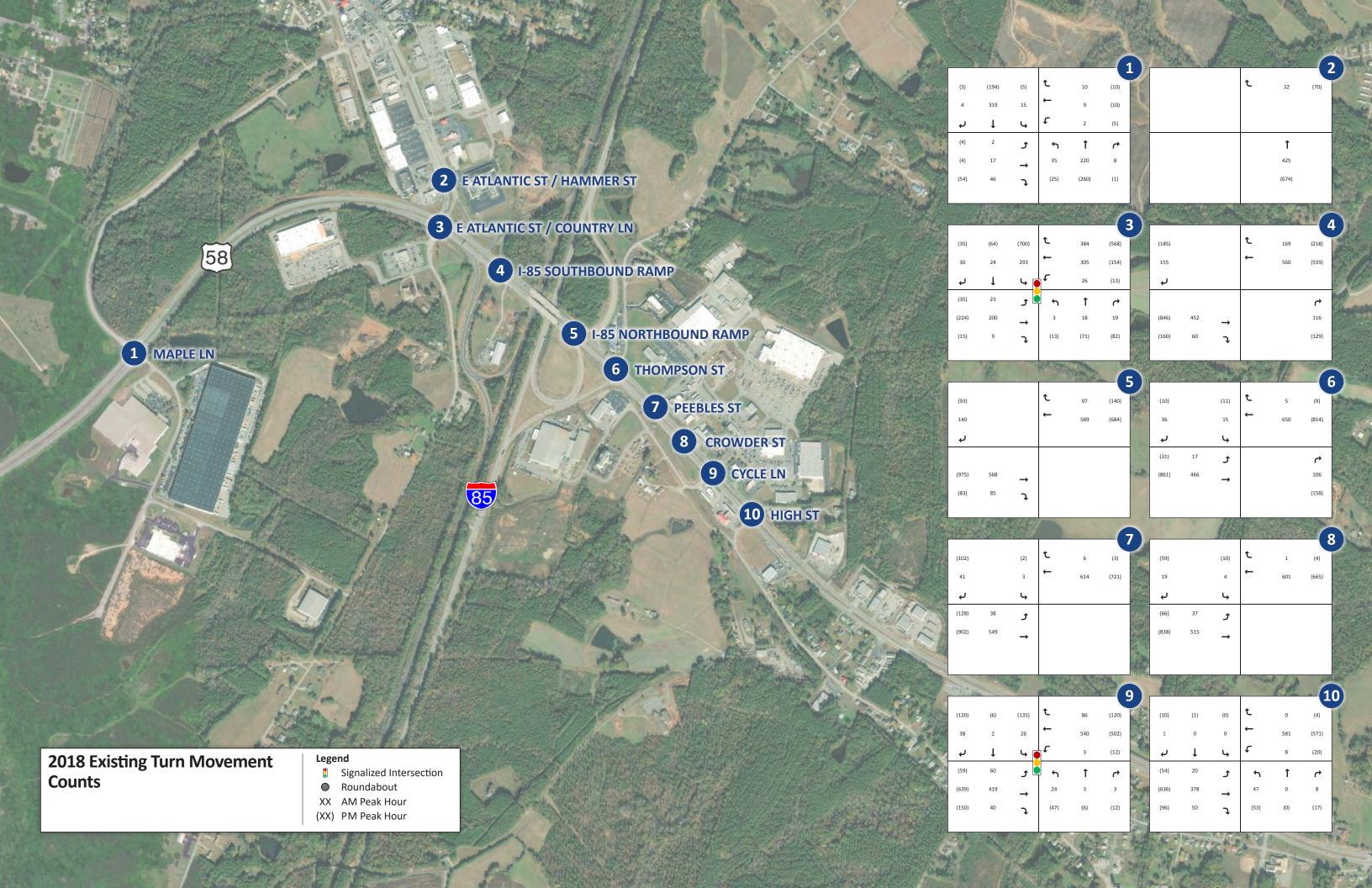
- Town of South Hill Overview
- Intersection of US 58 and Country Lane
- US 58 and I-85 Interchange
- US 58 Eastern Corporate Limits: US 58 intersections between Thompson Street and High Street

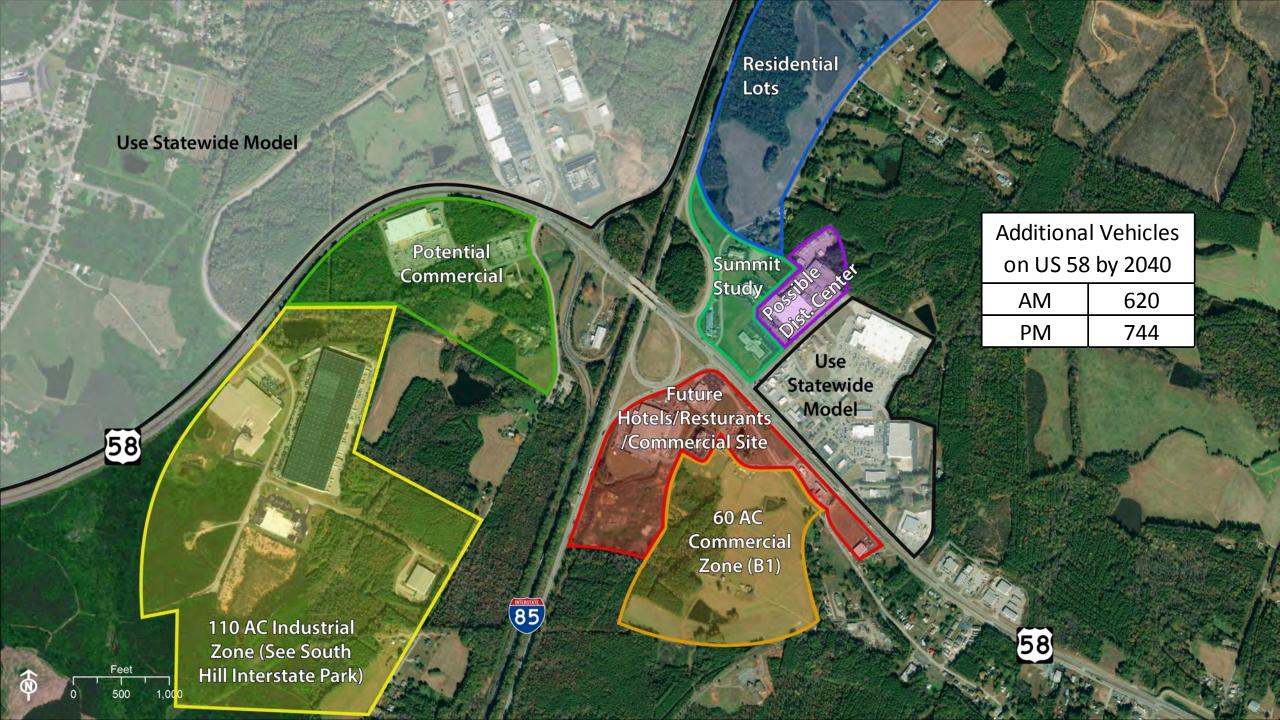
Independent Utility Considerations:

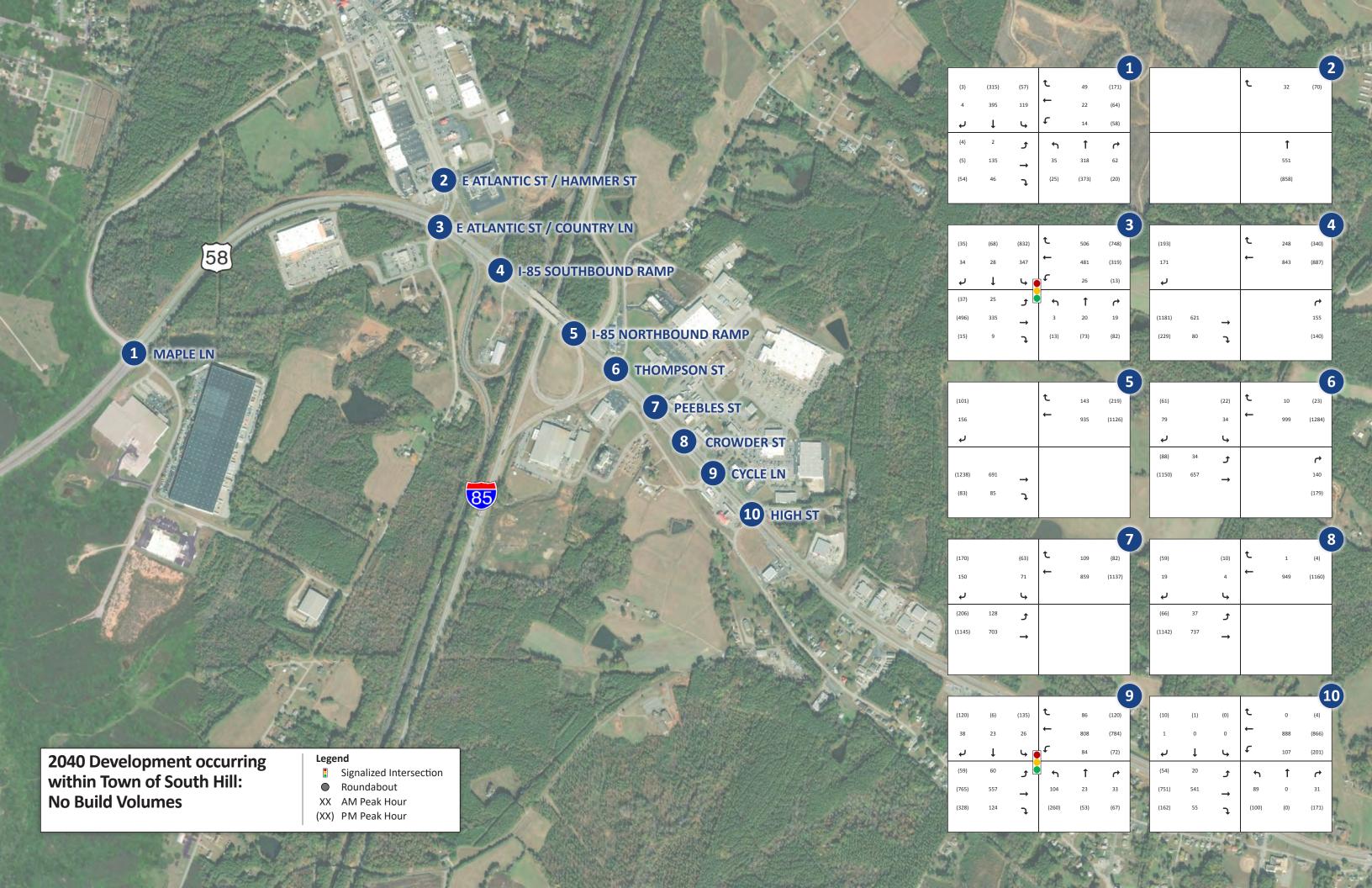
- Diverging Diamond Interchange with Town of South Hill Recommendations
- Roundabouts Interchange with Town of South Hill Recommendations

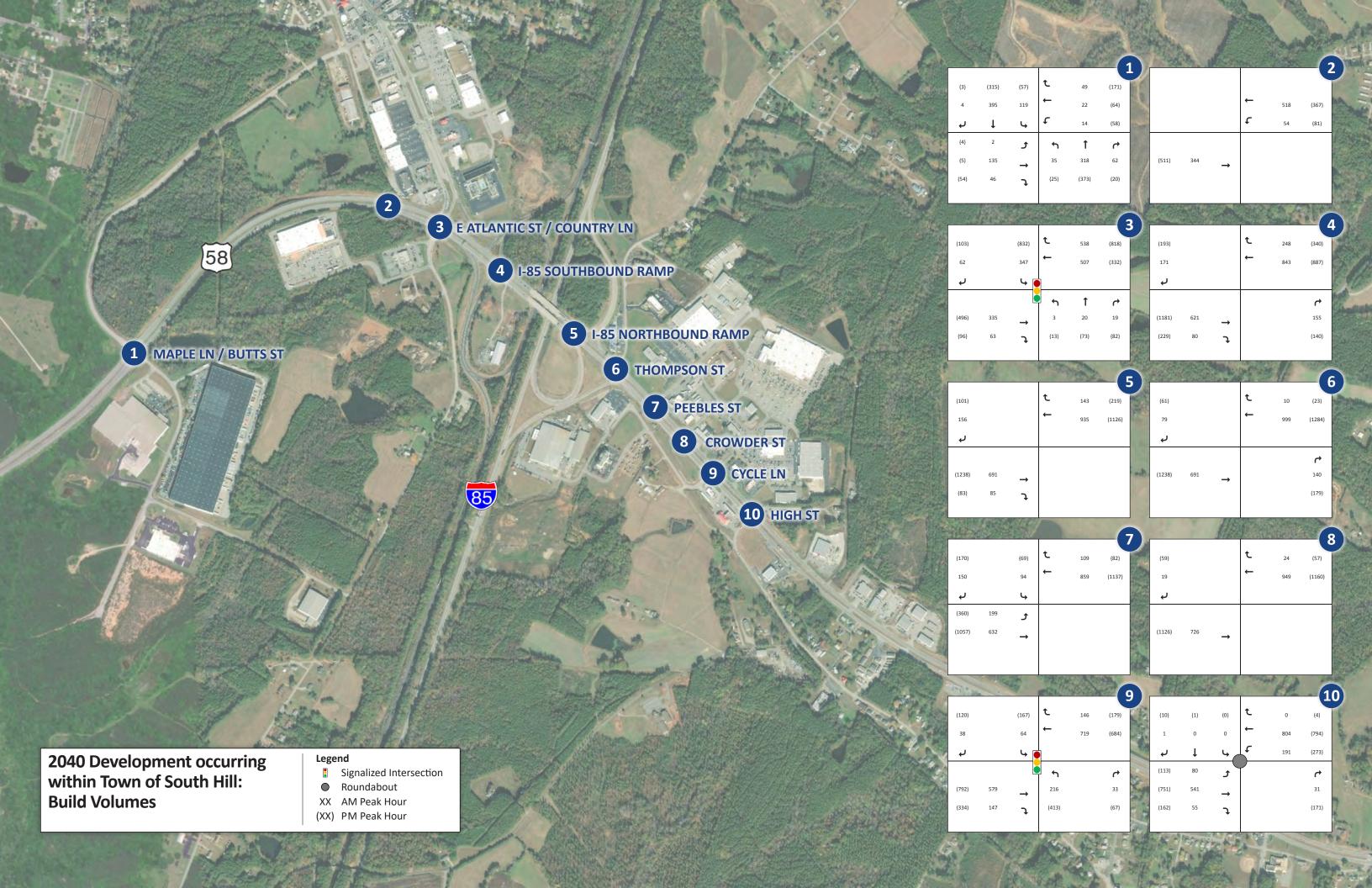


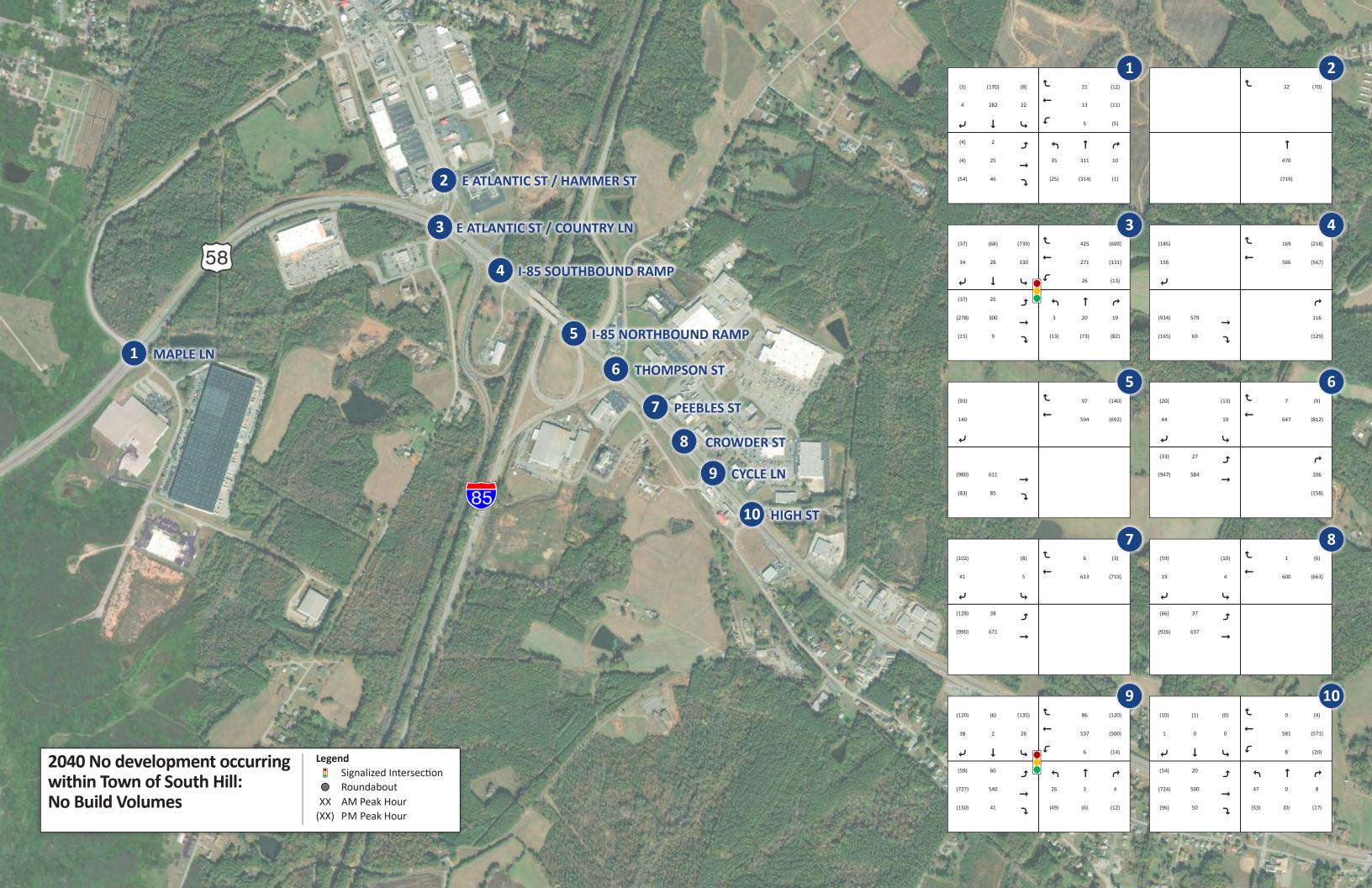


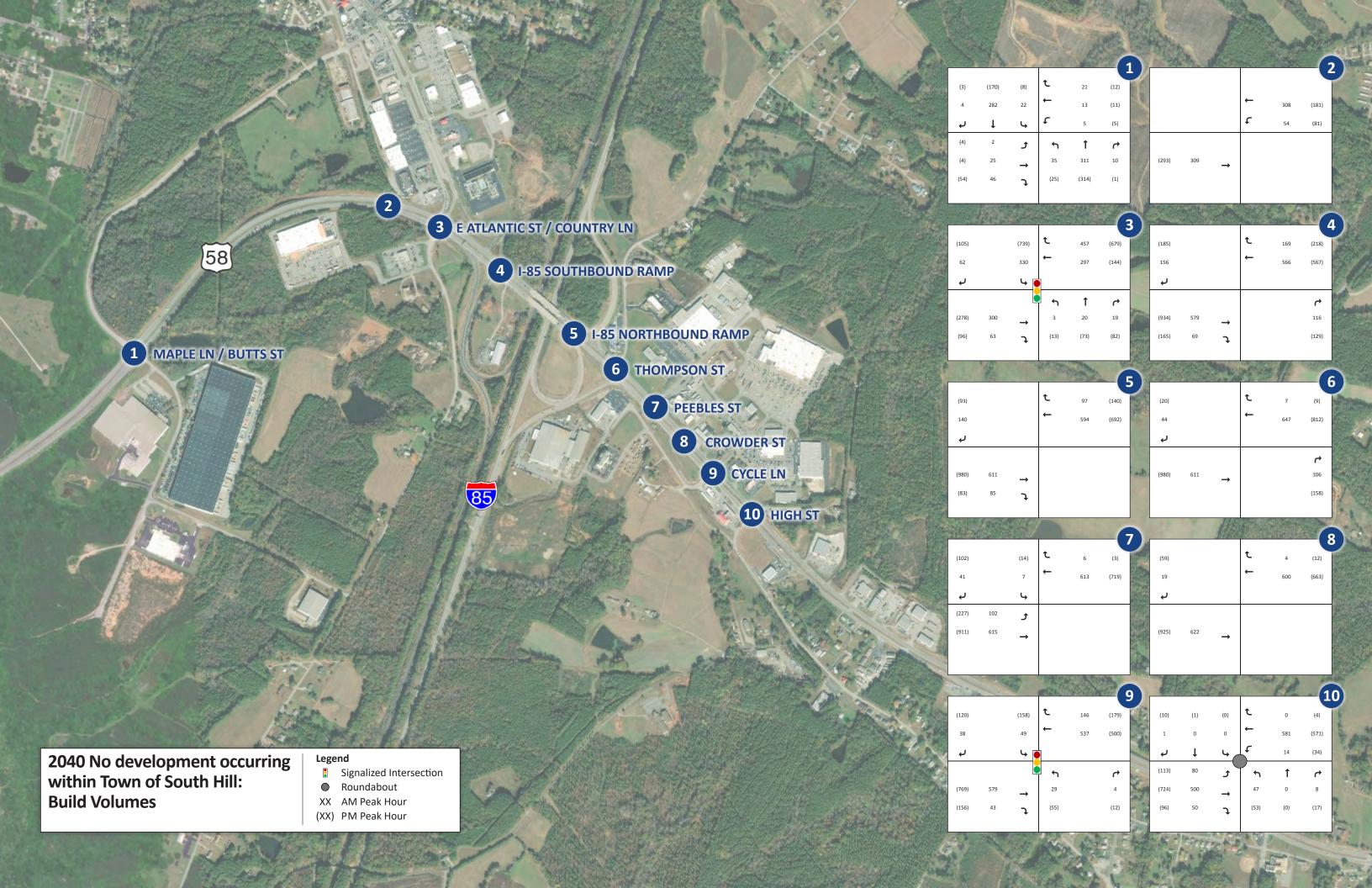


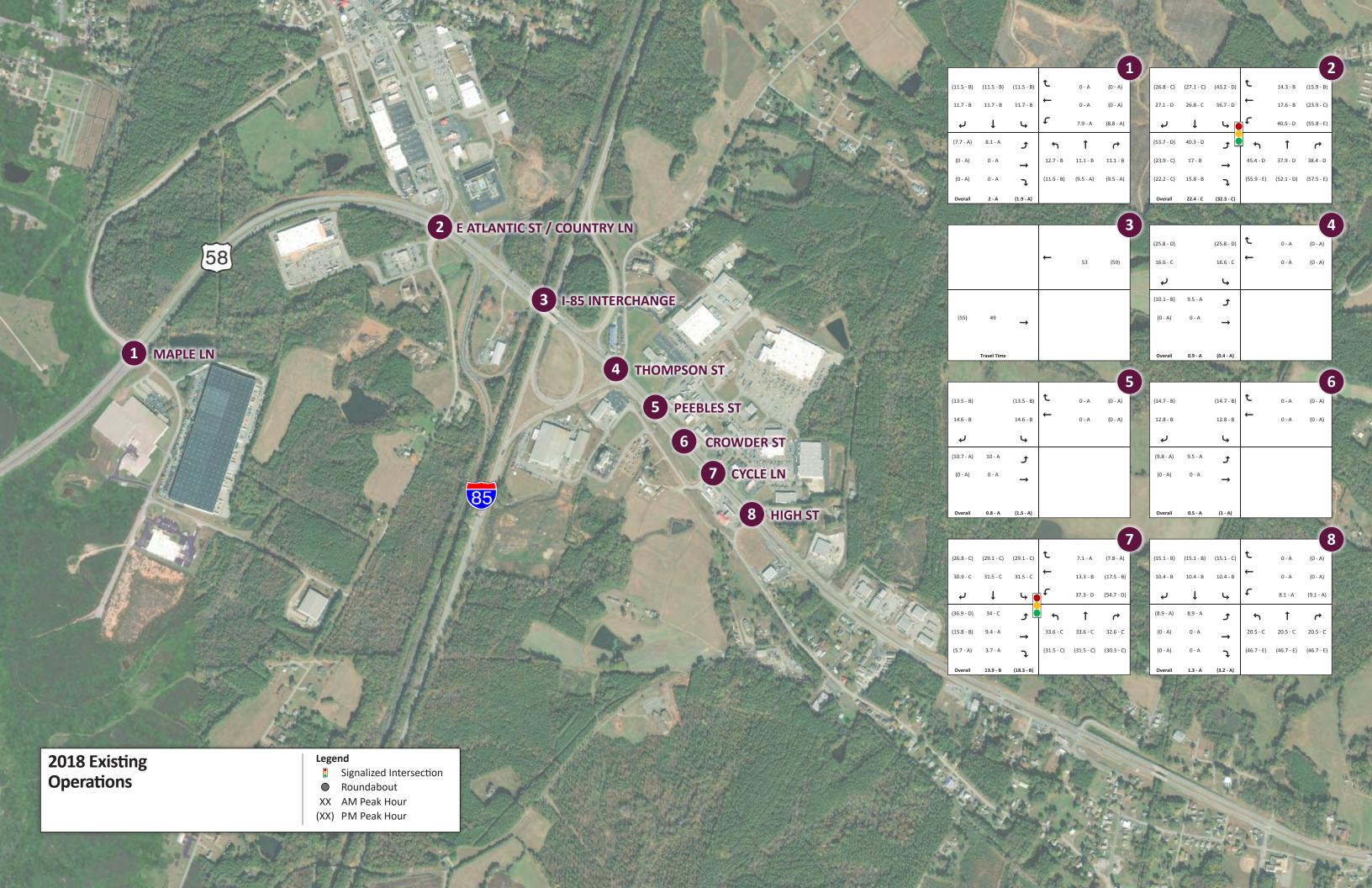


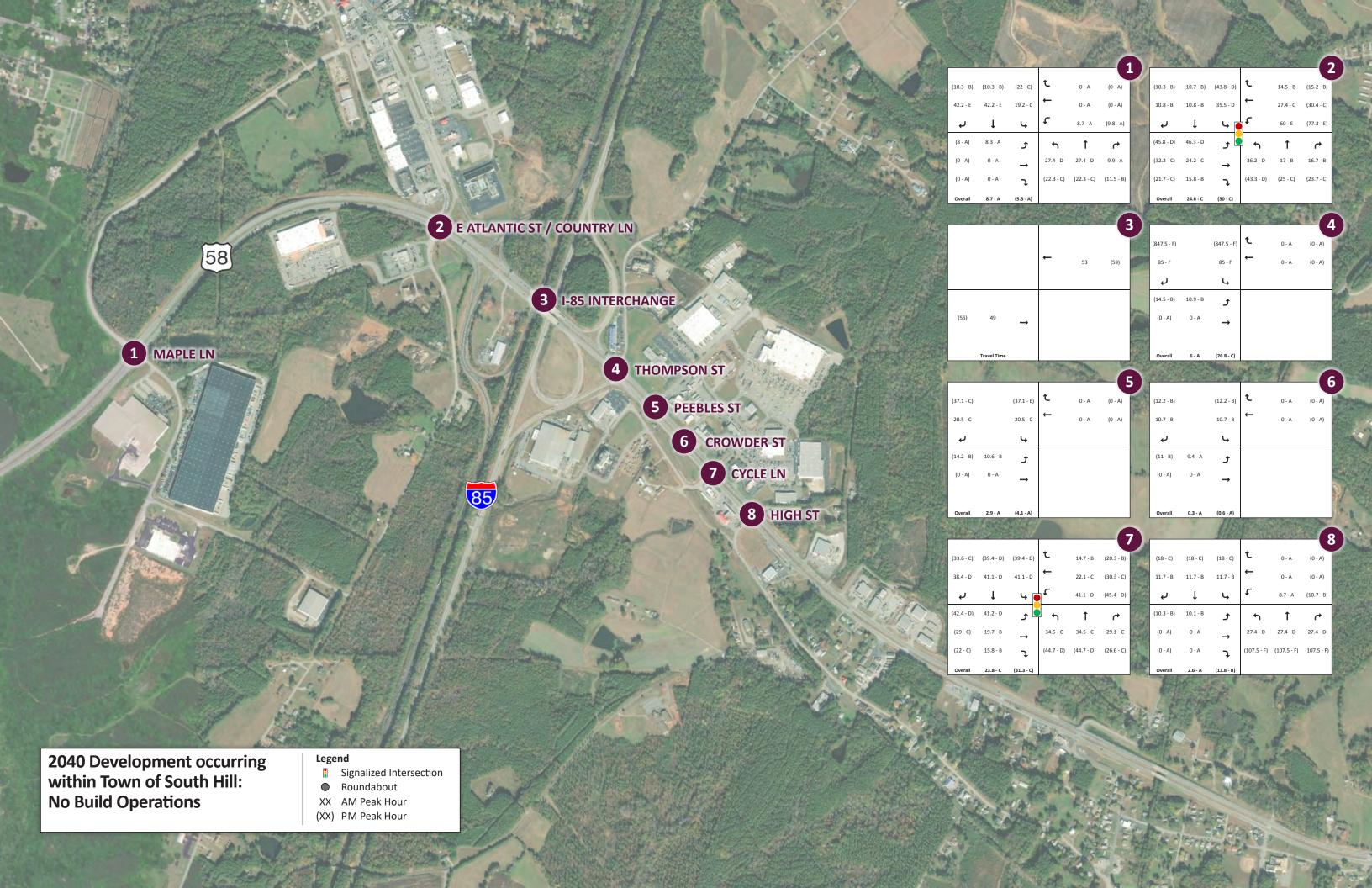


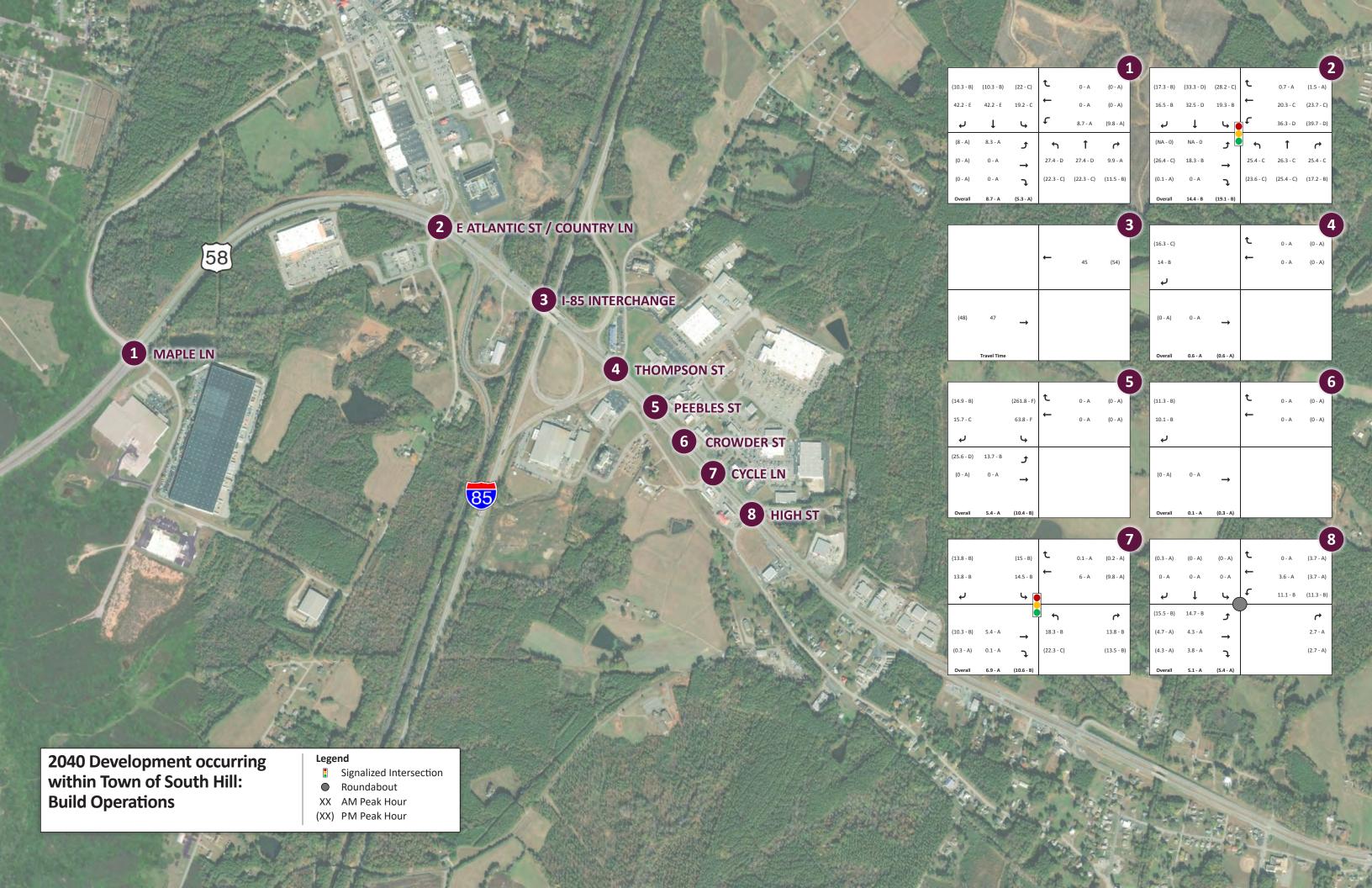


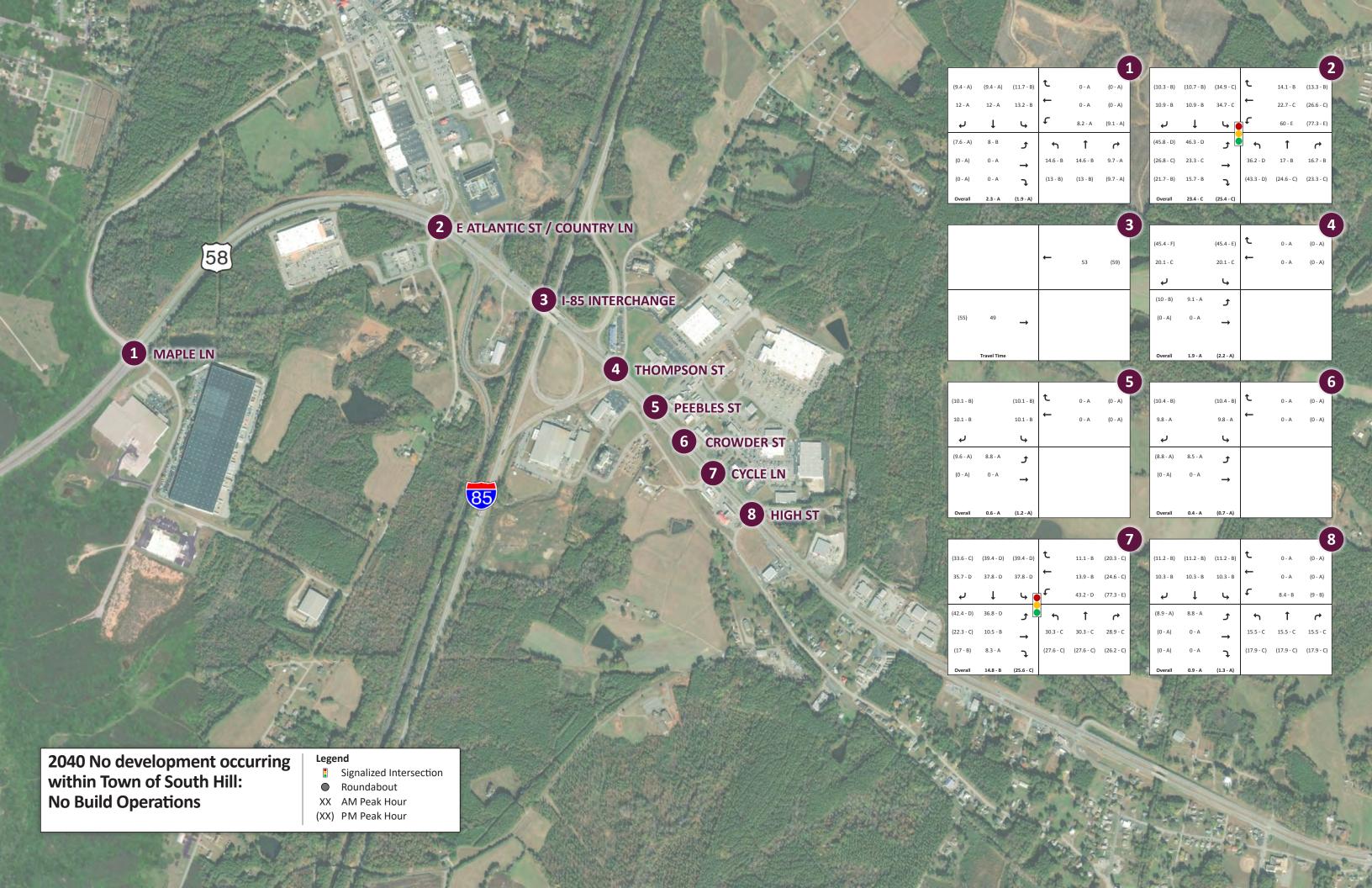


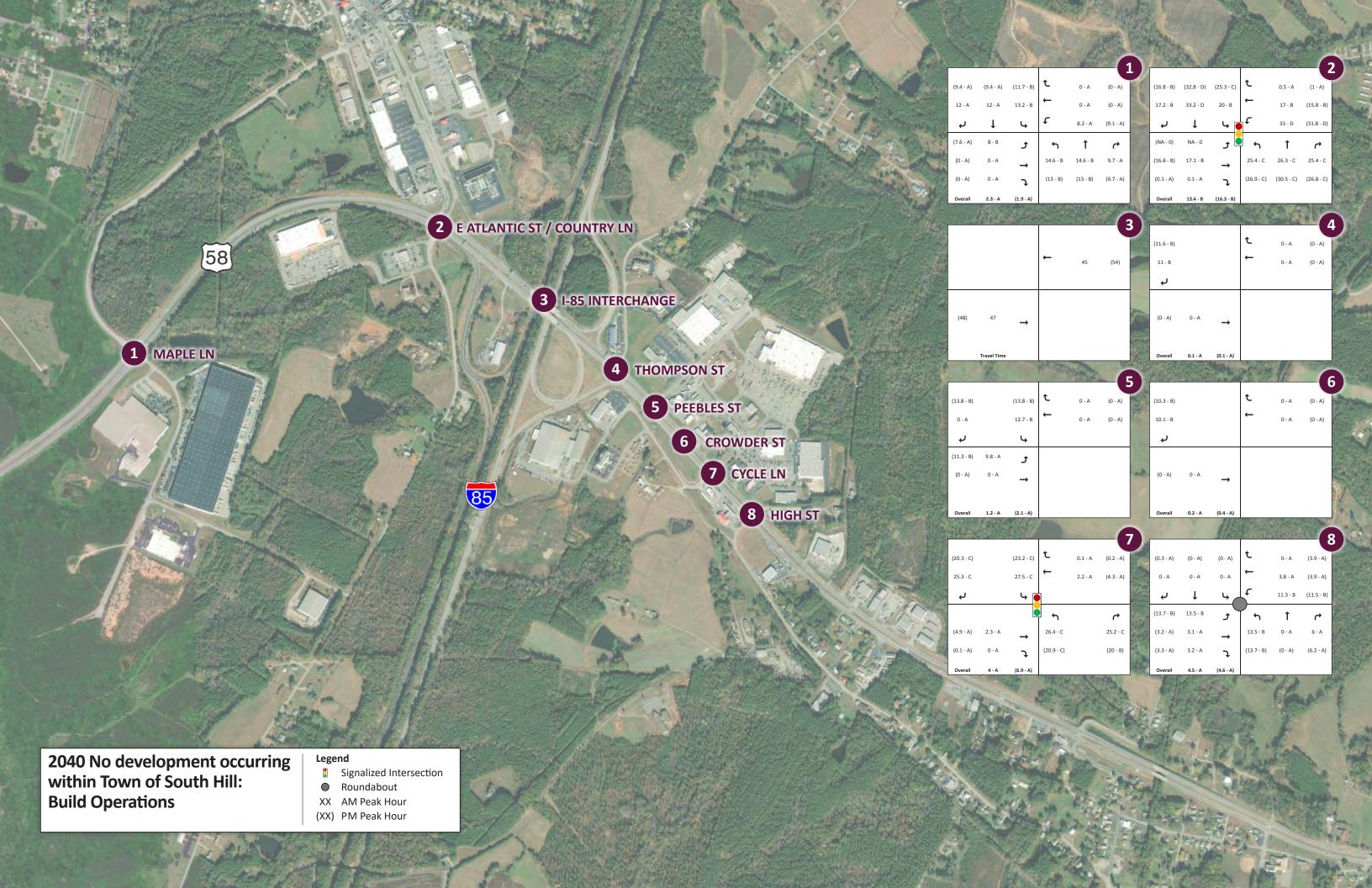




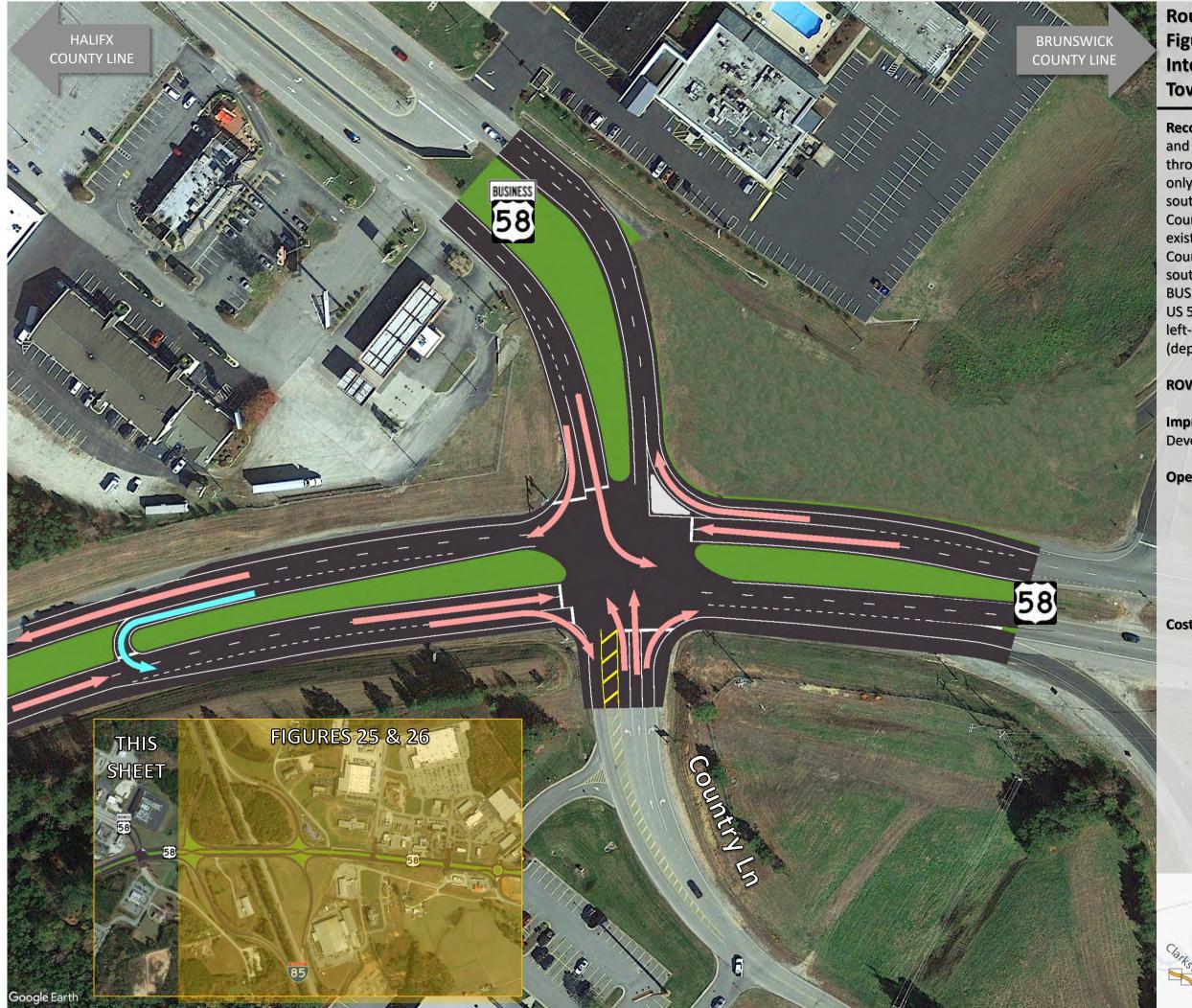












Route 58 Arterial Management Plan Figure 24 Intersection #49: US 58 with Country

Intersection #49: US 58 with Country Ln
Town of South Hill

Recommendation: Reconfigure the existing intersection and traffic signal to a three-phase signal. Permit only through and right-turn movements on US 58. Permit only left and right-turn movements from US 58 BUS southbound onto US 58 and full movements from Country Ln northbound. Construct U-turn area west of existing intersection to permit movements destined to Country Ln from US 58 westbound or US 58 BUS southbound. Remove existing I-85 off ramp onto US 58 BUS and construct continuous flow right-turn lane from US 58 westbound onto US 58 BUS. Eastbound US 58 left-turns to be managed at Maple Lane or interchange (depending on interchange configuration).

ROW Impacts: All improvements are within the ROW

Improvement Type: Congestion Mitigation, Economic Development, Safety, Travel Time Preservation

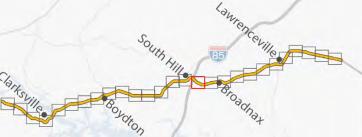
Operations:

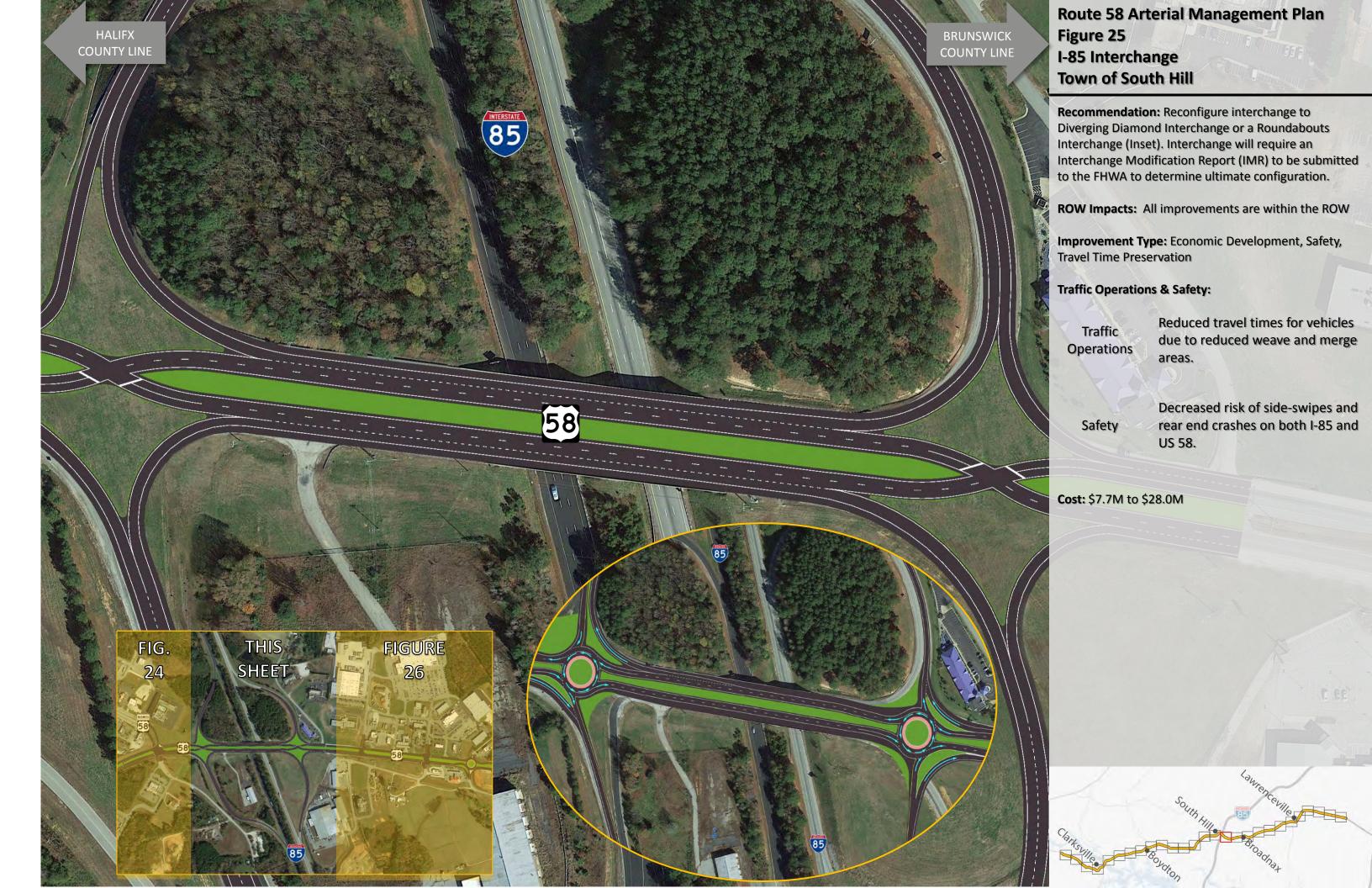
2040 Future Delay (sec - LOS)	No Build	Build
AM	24.6-C	17.4-C
PM	30.4-C	20.4-C

Cost: \$1.9M to \$3.1M



Standard Movements
Re-routed Movements







Route 58 Arterial Management Plan Figure 26 US 58 Eastern Corporate Limits Town of South Hill

Interim Recommendation: Reconfigure Thompson St intersection to right-in/right-out only, improve storage length of eastbound US 58 left-turn lane onto Peebles St, reconfigure Crowder St intersection to right-in/right-out only, reconfigure Cycle Lane to a two-phase signal, and construct a roundabout at the intersection of High St. Construct inter-parcel connections to maintain access between Thompson St and Peebles St, and between Cycle Ln and High St. Town maintained streets should be investigated further to determine pavement condition and capacity improvements to maintain efficient traffic flow.

Long-term Recommendation: As development occurs, additional improvements will be needed at the intersection of Peebles St and US 58. These improvements may require reviews and approvals by district, state, and FHWA officials. The roundabout at High St will need to be reconfigured to remove northbound left and thru movements to maintain capacity of the corridor.

ROW Impacts: All improvements on US 58 are within the ROW. Inter-parcel connections and Town maintained street improvement may require significant ROW acquisition.

Improvement Type: Economic Development, Safety, Travel Time Preservation

Traffic Operations

2040 Travel Times (min)	Fastbound	US 58	Westbour	nd US 58
Condition	No Build	Build	No Build	Build
AM	1:02	0:54	1:02	0:58
PM	1:32	1:17	1:38	1:26

US 58 Improvements: \$6.4M to \$8.3M Town Street Improvements: \$1.6M to \$10.0M



Standard Movements

